

# A Mathematical Model of Phosphorus in Completely Mixed Lakes with Special Application to Lake Warner, Massachusetts

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## ABSTRACT

The dynamic response of Lake Warner (Hadley, Massachusetts) to pollution abatement was studied by field measurements and mathematical modeling. Stream flow and phosphorus concentration measurements were used to describe phosphorus flow through the lake system. The lake system released phosphorus, presumably from the sediments, during July 1981, but retained phosphorus during the fall of 1981. Seepage through lake sediments was measured using seepage meters and was determined to be insignificant in the water and phosphorus budget of Lake Warner.

Differential equations for the lake water, sediment interstitial water and sediment solid phase phosphorus concentration developed by others in previous studies were solved using a trial function method. Approximate solutions were developed using steady state and pseudo-order assumptions. The analytic solution was programmed on a hand-held calculator. Sensitivity analysis was carried out for model parameters. The analytic expression for the solution to the differential equations describing lake recovery was an important tool for lake planning, management and research purposes.

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## NOMENCLATURE

Symbol	Dimensions in Mass (M), Length (L) and Time (T).
A	Area ( $L^2$ ), square meters
C	Lorenzen (1973), Average Annual Nutrient Concentration ( $M/L^3$ ), grams per cubic meter
$C_s$	Lorenzen (1973), Nutrient Concentration in Sediment ( $M/L^3$ ), grams per cubic meter
$D_r$	Sediment Reactive Depth (L), meters
$K_1$	Release Rate (L/T), meters per day
$K_2$	Sedimentation Rate (1/T), per day
$K_3$	Conversion Rate (1/T), per day
k	Lorenzen (1973), Net Specific Rate of Loss to Sediment (M/T), meters per year
$k_1$	Lorenzen (1973), Specific Rate of Nutrient Transfer to Sediments (M/T), meters per year
$k_2$	Lorenzen (1973), Specific Rate of Nutrient Transfer from Sediment (M/T), meters per year
M	Mass Flow In from all Sources (M/T), grams per year
$P_i$	Sediment Interstitial Total Phosphorus Concentration ( $M/L^3$ ), micrograms per liter
$P_{i0}$	Sediment Interstitial Phosphorus Concentration Initial Condition ( $M/L^3$ ), micrograms per liter
$P_{i\infty}$	Sediment Interstitial Phosphorus Concentration, Steady State ( $M/L^3$ ), micrograms per liter
$P_L$	Lake Water Total Phosphorus Concentration ( $M/L^3$ ), micrograms per liter
$P_{L0}$	Lake Water Total Phosphorus Concentration Initial Condition ( $M/L^3$ ), micrograms per liter
$P_{L\infty}$	Lake Water Total Phosphorus Concentration, Steady State ( $M/L^3$ ), micrograms per liter

$P_s$	Sediment Solid Phase Phosphorus Concentration ( $M/L^3$ ), micrograms per liter
$P_{so}$	Sediment Solid Phase Phosphorus Concentration, Initial Condition ( $M/L^3$ ), micrograms per liter
$P_{s\infty}$	Sediment Solid Phase Phosphorus Concentration, Steady State ( $M/L^3$ ), micrograms per liter
$Q$	Average Annual Flow Rate ( $L^3/T$ ), cubic meters per day
$R_{rel}$	Rate of Release from Lake Bottom ( $M/L^3-T$ ), micrograms per liter per day
$R_{sed}$	Rate of Sedimentation to Lake Bottom ( $M/L^3-T$ ), micrograms per liter per day
$r$	Root of cubic equation ( $1/T$ ), per day
$T$	Hydraulic residence time (T), days
$u$	Macro-Seepage Velocity ( $L/T$ ), micrometers per second
$V$	Volume ( $L^3$ ), liters
$V_L$	Lake Water Volume ( $L^3$ ), cubic meters
$V_s$	Volume of Sediment ( $L^3$ ) cubic meters
$X_1 \dots X_7$	Lake Model Parameters ( $1/T$ ), per day
$X_8$	Lake Model Parameter ( $M/L^3-T$ ), micrograms per liter per day
$Z$	Lake Mean Depth (L), meters
$\epsilon$	Sediment Porosity
$\alpha, \beta$ and $\gamma$	Lake Model Parameters ( $1/T$ ), per day

## CHAPTER I

## INTRODUCTION

The eutrophication of many freshwater lakes has been accelerated by cultural activities such as wastewater overflows and nonpoint pollution. Lake restoration, funded under Section 314 of the Federal Water Pollution Control Act Amendments of 1972, includes the diversion of wastewater overflows. After such diversions or other water pollution control measures, lake sediments can act as a nutrient source for biological activity and, therefore, control the rate of recovery of a eutrophic lake.

Lake nutrient budget models can be used by planners and engineers to predict a lake's response to pollution control efforts. When constructing a nutrient budget for a lake one must account for all nutrient sources, such as lake sediments and ground water discharge/recharge, which may effect the recovery of a eutrophic lake.

The recovery of Lake Warner (Massachusetts) was the subject of past studies by Snow and DiGiano (1976) and Bingham and Feng (1980). Wastewater overflows were diverted from Lake Warner in 1975, thus making it a convenient site for testing the lake recovery model developed by Snow and DiGiano (1976). This model predicted that it would take about 14 years for Lake Warner to recover from its eutrophic state. Subsequent field work by Bingham and Feng (1980) showed that Lake Warner had recovered in

just one year. Hence, they modified the Snow and DiGiano model to include a reaction rate ( $K_3$ ) which describes the conversion of sediment solid phase phosphorus to sediment interstitial phosphorus. The value of this reaction rate may control the recovery of a eutrophic lake when sediments contain a significant reservoir of phosphorus.

### Objectives

The two major goals of this research were to examine the significance of ground water-lake interactions and to seek an analytical solution to the ordinary differential equations describing phosphorus in the lake recovery model. The hypothesis that ground water may act as a transport mechanism for recycling lake sediment nutrients into the lake water was tested by measuring the ground water discharge/recharge through Lake Warner sediments using seepage meters. It was also of interest to ascertain lake sediment phosphorus release through a mass balance on lake water phosphorus.

An analytical solution to the lake recovery model was desired to eliminate the need for a high speed computer used to implement the numerical solutions of past studies. Simplifications of the governing differential equations using approximation techniques were studied in an effort to produce a compact model

for use in lake planning, management and research purposes. A simplified model was deemed acceptable if predictions closely matched the results of the full analytical solution.

Sensitivity analysis was carried out on model parameters to determine the effect variations in the values used have on model predictions. This information can be used to show which parameters must be estimated very accurately and which parameters can be approximated without significantly affecting model predictions.

### Scope

Many lakes in Massachusetts are similar to Lake Warner in that they are relatively shallow, have short hydraulic residence times and can be treated as being completely mixed for time periods greater than a few hydraulic residence times. Many of these lakes were formed by man diking a stream. This study is concerned with the phosphorus budget in this category of lakes. Lake Warner will be used as an illustrative case study. Quabbin Reservoir illustrates a man-made reservoir whose phosphorus budget is unlikely to be closely described by the methods illustrated in this study.

The role of ground water in affecting the phosphorus budget of a lake has not been clearly illustrated in past studies of Lake Warner. This study reviews literature on observations of

ground water-lake interactions and uses field measurements for assessing the importance of ground water to Lake Warner's water and phosphorus budgets. Field measurements on the Mill River phosphorus loading and on the lake water are used to evaluate the release of phosphorus from Lake Warner sediments. An analytical solution to the Bingham and Feng (1980) mathematical model of the recovery of a eutrophic lake is presented. Equations for the lake water phosphorus, sediment interstitial phosphorus and sediment solid phase phosphorus are presented. These equations provide tools which engineers and scientists can use to determine the values of reaction rates which govern phosphorus cycling in shallow lake systems.

Sensitivity analysis is used to show the effects on model predictions due to marginal variations in input parameters.

## C H A P T E R I I

## LITERATURE REVIEW

This section reviews literature on phosphorus chemistry in lake environments. Lake phosphorus budget modeling and mathematical methods for solving the resulting equations are examined. The importance of groundwater seepage in the water and nutrient budgets of lakes is discussed.

Phosphorus Chemistry

Carbon, nitrogen and phosphorus are required by plant life in ratios of approximately 100:15:1, respectively. Phosphorus has no gaseous phase and is relatively scarce, therefore making it a limiting nutrient to plant growth in most natural aquatic ecosystems. Phosphorus in nature generally occurs as phosphate in both dissolved and solid phases. Categories of phosphate can be operationally defined by particle size and acid molybdate reactivity.

Because of complex limnological transformations in lake systems (see Figure 1) dissolved phosphate concentrations are not a good index of the biologically available phosphorus during the growing season. Schindler et al. (1971) claim that the amount of phosphorus supplied rather than the amount in solution at any



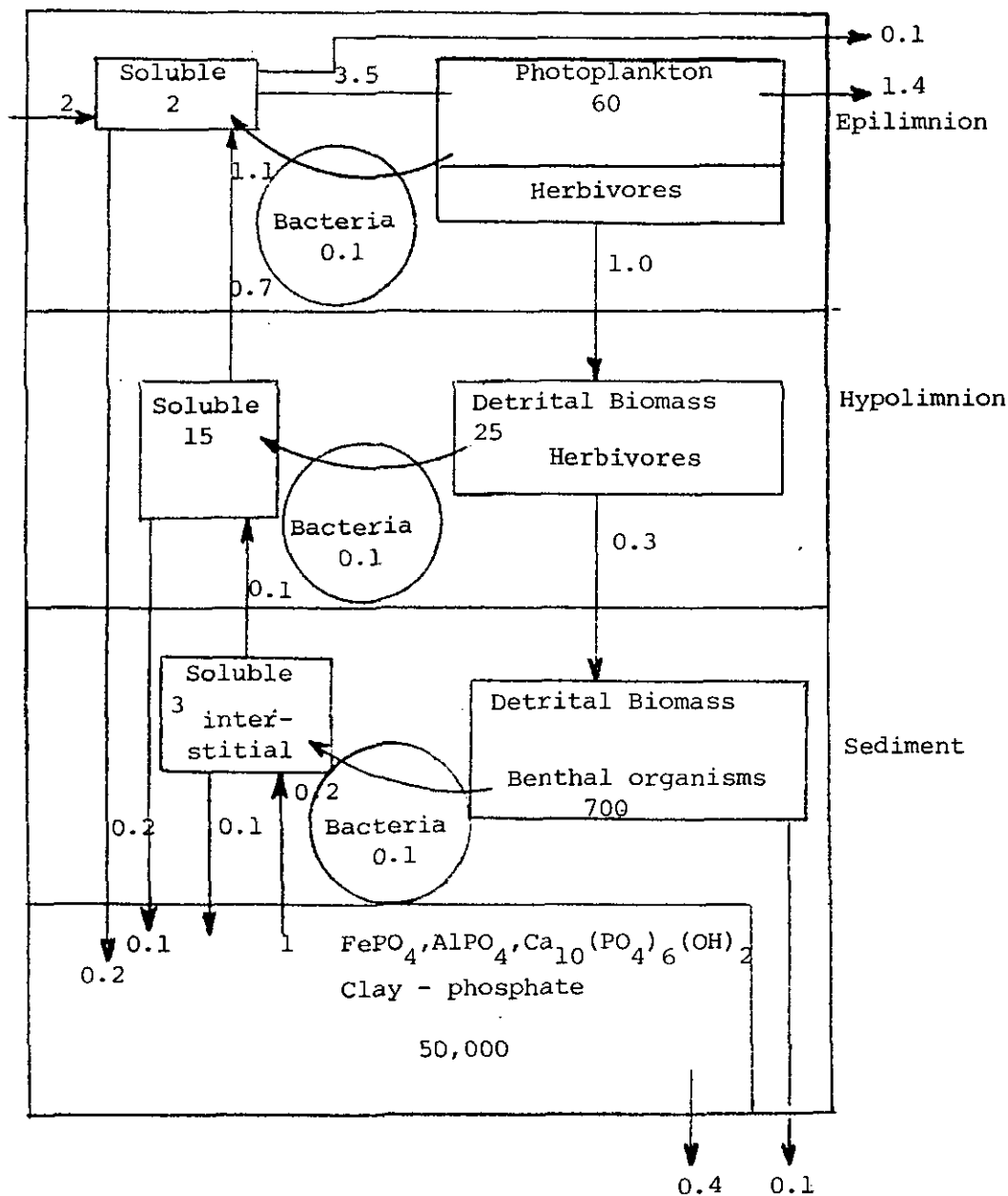


Figure 1. Transformations of phosphorus in a hypothetical lake system. This steady state model shows representative phosphorus concentrations for model compartments ( $\mu\text{g}/\text{l}$ ) and numbers on arrows show mass flow rates ( $\mu\text{g}/\text{l-day}$ ) (Stumm and Leckie, 1971).

point in time must be known to predict algal growth. Lee et al. (1978) note that for most water bodies the biologically available phosphorus is between the total phosphorus and soluble orthophosphate loading since some of the particulate phosphorus is solubilized (Stumm and Morgan, 1981).

The concentration of dissolved phosphate in natural waters gives little indication of phosphate availability for biological growth. The residence time of dissolved phosphate in natural waters ranges from 0.0021 to 8.3 days (Stumm and Morgan, 1981). Lean (1973) found that fifty percent of  $^{32}\text{PO}_4$  was taken up by algae in less than two minutes. Schindler et al. (1971) found that dissolved phosphate concentrations were undetectable within an hour after artificially raising the phosphate concentration of a lake to 10 g/l. Dissolved inorganic phosphate concentrations in water fluctuate on a daily cycle (Vollenweider, 1968). Low dissolved phosphate concentrations were found at night and higher phosphate concentrations were found during the day. It was suggested that phosphates were required by phytoplankton for energy conversion at night and that some phosphates were released by growing cells during the day.

Lake bottom phosphorus occurs in the sediment solid phase and in the sediment interstitial water. Phosphate adsorption by lake sediments is governed by sediment composition and by physical and chemical conditions in the sediment such as redox potential, pH and temperature. Under anoxic lake bottom

conditions (which might occur during summer stagnation periods) a lower redox potential in the sediment might develop causing an increase in the dissolution of Fe bound phosphorus (Ku et al., 1978). Also the phosphorus binding capacity of sediments is greater as pH decreases. Figure 2 shows the effect of pH on Fe bound phosphate. It is evident that the dissolution of sediment phosphorus would increase in response to high pH levels which would occur during summer algal blooms.

Many researchers believe that phosphorus release from lake bottoms is from sediment interstitial phosphorus (Syers et al., 1973). Hence the dissolution of sediment solid phase phosphorus to sediment interstitial phosphorus may govern the release of phosphorus to the lake water.

Phosphorus released as orthophosphate from lake bottoms undergoes rapid transformation into other more stable compounds which are components of total phosphorus. Hence, when measuring release rates (such as Snow and DiGiano's  $K_1$ ) total phosphorus is used as a measure of the phosphorus released from lake bottoms.

#### Lake Phosphorus Budget Modeling

Lakes which are phosphorus limited exhibit good correlations between total phosphorus concentration and primary productivity parameters such as chlorophyll a (Williams et al., 1978; Dillon and Rigler, 1974; Jones and Bachmann, 1976).

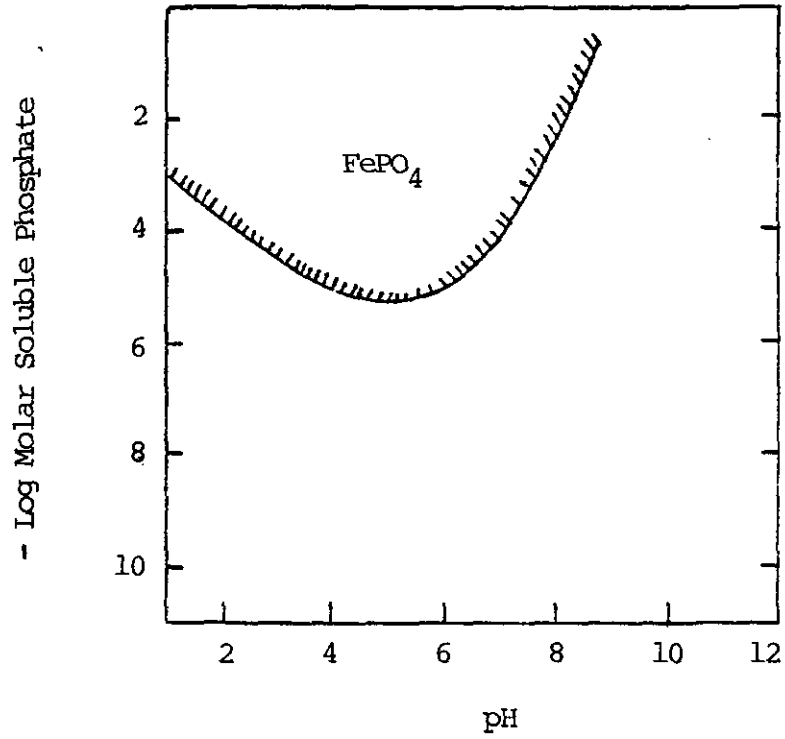


Figure 2. Solubility diagram for iron phosphate (after Stumm and Leckie, 1971).

Phosphate sources to aquatic systems result from natural weathering and dissolution of phosphate minerals, soil erosion, soil fertilization, biological transfer, detergents and domestic and industrial wastewaters (Stumm and Morgan, 1981). Table 1 shows phosphorus export coefficients based on the U.S. Organization for Economic Cooperation and Development (OECD) eutrophication study.

Vollenweider (1968) developed an empirical model which relates total phosphorus loading to a lake's trophic state. When areal phosphorus loading ( $L(P)$ ) was plotted against mean depth ( $Z$ ) on a log-log plot (figure 3), lakes appear to be grouped according to their trophic states. Lines dividing trophic states define critical phosphorus loading boundaries. A line dividing the oligotrophic and eutrophic states has the equation

$$L_c(P) \text{ (mg/m}^2\text{-yr)} = (25 \text{ to } 50)Z^{0.6} \quad 2.1$$

Dillon (1975) noted that Cameron Lake appeared in the eutrophic zone in this type of plot but did not exhibit the normal characteristics of eutrophy due to a short hydraulic residence time. Vollenweider (1976) refined his criterion axis to include the hydraulic residence time ( $T$ ). The resulting log-log plot (figure 4) of areal phosphorus loading vs  $Z/T$  gives a more widely applicable model of lake trophic status. The line dividing oligotrophic from eutrophic lakes has the equation:

Table 1. Representative watershed total phosphorus export coefficients from the U.S. OECD eutrophication study (after Lee et al., 1978).

Land use	Total Phosphorus (g/m <sup>2</sup> /yr)
Urban	0.1
Rural/Agruculture	0.05
Forest	0.01
Other:	
rainfall	0.02
dry fallout	0.08

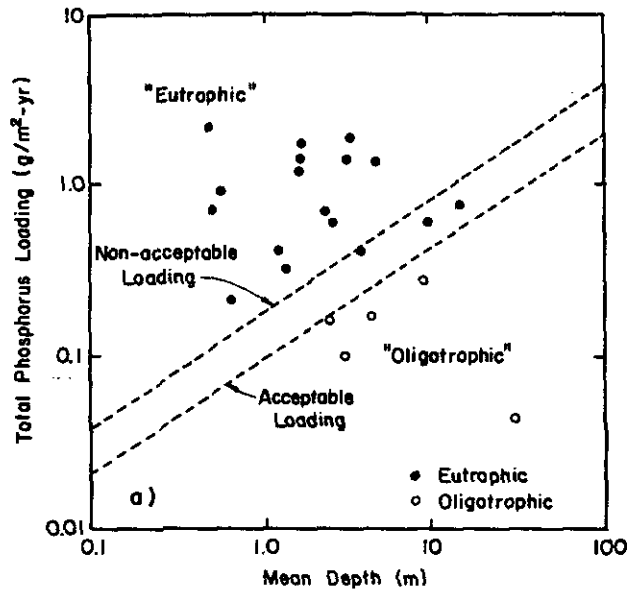


Figure 3. Vollenweider (1968) phosphorus loading - trophic state criterion axes.

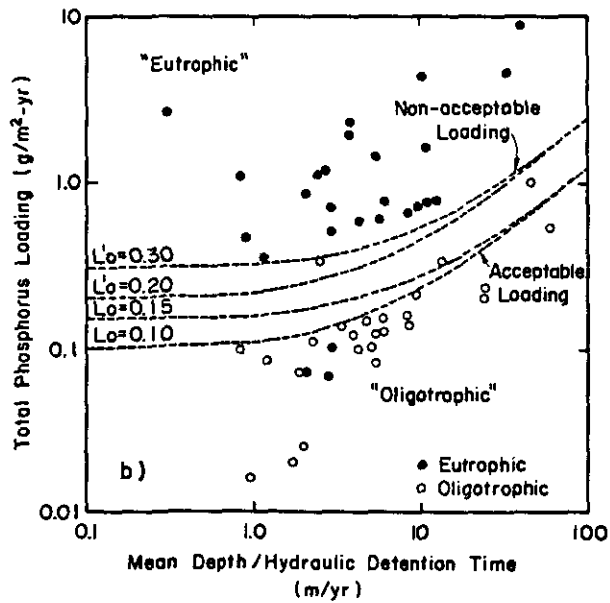


Figure 4. Vollenweider (1976) phosphorus loading - trophic state criterion axes.

$$L_c(P) = 100 (Z/T)^{0.5} \quad 2.2$$

Vollenweider's phosphorus loading-trophic state criterion axes are useful for establishing maximum allowable phosphorus loadings to lakes. More complex mass balance models of phosphorus dynamics in lake systems are needed to predict the time dependent response of lake phosphorus concentrations to pollution control measures.

Lorenzen (1973) summarizes model derivations which describe the fate of a conservative substance (total phosphorus is the nutrient of concern) in a well mixed lake. Model I (schematically shown in Figure 5) allows for inflow, outflow and sedimentation of phosphorus. Continuous Flow Stirred Tank Reactor (CFSTR) assumptions allow the lake to be treated as completely mixed. Model II (Figure 5) considers the release of phosphorus from the lake bottom. It is assumed that the concentration of phosphorus in the lake bottom does not change over time. Model III (Figure 5) allows for depletion of phosphorus in the lake bottom over time.

Snow and DiGiano (1976) developed a lake nutrient budget model which differs from the Lorenzen Model III in that lake bottom phosphorus release is from the sediment interstitial water. A linear relationship was developed empirically which described the equilibrium concentration of sediment solid phase phosphorus and sediment interstitial phosphorus. This



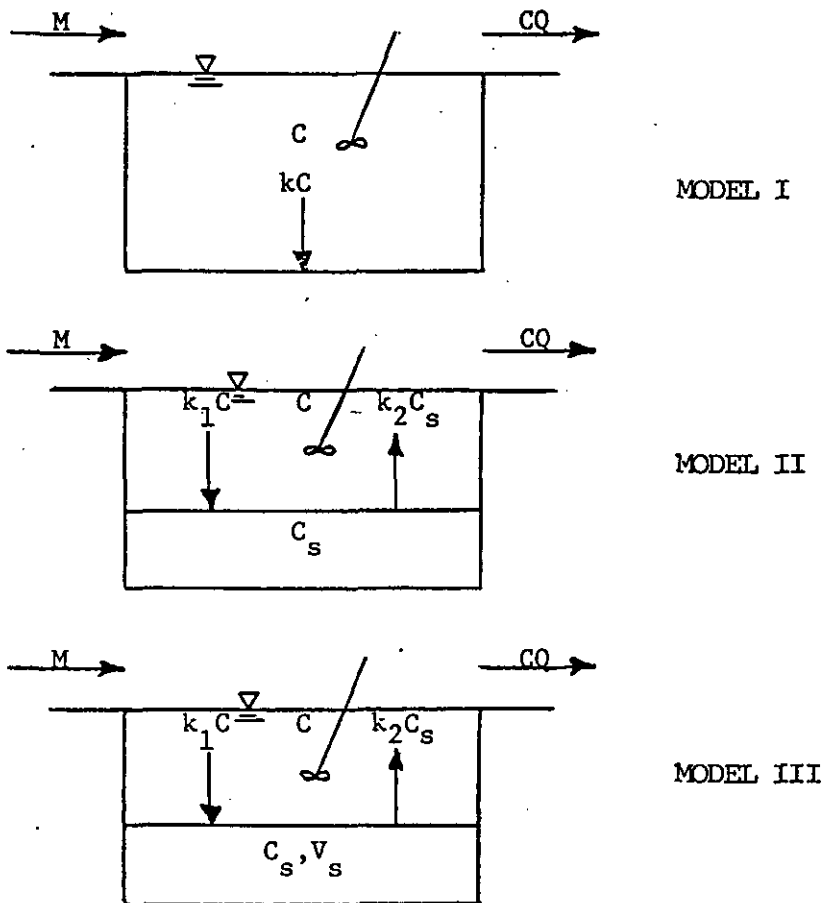


Figure 5. Schematic representation of mass balance models derived by Lorenzen (1973).

assumption, in effect, allowed an instantaneous conversion rate between sediment solid phase and sediment interstitial phosphorus. The Snow and DiGiano model predicted that it would take Lake Warner about 14 years to recover from its eutrophic state in response to a reduction in phosphorus loading. Bingham and Feng (1980) found through field observations that Lake Warner had recovered from its eutrophic state (or reached equilibrium with the reduced phosphorus loading rate) one year after the diversion of wastewater overflows. Hence, they modified the Snow and DiGiano model to include a rate constant which describes the conversion from sediment solid phase to sediment interstitial phosphorus. There is no experimental evidence for choosing a particular value for this rate constant and it is found by model calibration.

#### Mathematical Methods

The most complex Lorenzen mass balance model (Model III) consists of two simultaneous ordinary differential equations with constant coefficients. The equations can be solved numerically or by analytical methods (Lorenzen, 1976). The solution of three simultaneous linear first order ordinary differential equations with constant coefficients is required by the modified Snow and DiGiano model and will be considered in the following discussion on mathematical methods. Methods of solving the equations

include the Laplace transform method, the eigenvalue method, the power series method, the trial function method, numerical methods and other methods.

Laplace transform methods are convenient to use for solving the type of equations under consideration here. However, inverse transforms do not always exist (Churchill, 1958) and the solution of more than two simultaneous equations becomes formidable by this method. Similarly, eigenvalue methods (Kaplan, 1957) become complex when the solution of a system of three or more equations is attempted.

Manipulations of the equations can be aided by introducing matrix notation and solving the differential equations by numerical or power series methods (Himmelblau et al., 1968). The power series method (Rinaldi et al., 1979) is useful if the series converges quickly. -

The system of equations can be solved by first reducing them to a single equation of a higher order (Elsgolts, 1973). The solution of the higher order differential equation may be easier than the direct solution of the system of differential equations.

Bingham and Feng (1980) solved the lake model equations by numerical integration. The solution was carried out in one day time steps on a high speed computer.

A trial function method (Stockton, personal communication; Kaplan, 1957) is practical for solving a maximum of three

equations. A solution is postulated and unknown constants are found by manipulating algebraic equations. See Appendix B for details of the trial function method.

Order of magnitude analysis can be used to simplify systems of differential equations by focusing attention on the dominant terms and neglecting small terms. The method is most useful when the simplified equations are solvable whereas the original equations are intractable. One of the most widely recognized applications of the method was Prandtl's 1904 development of boundary layer theory to describe fluid motion near a solid surface (Lamb, 1932).

Approximation methods have been extensively applied in the study of chemical reaction kinetics. The pseudo-order approximation method (Freifelder, 1982), combines experimental planning with mathematical simplification to reduce the order of the differential equation describing a reaction. As rates of formation or consumption of reactants are proportional to the product of the concentration of the reactants raised to a power, the chemist is motivated to combine experimental and mathematical wizardry to transform intractable differential equations into those more amenable to solution. The method of pseudo-order approximation accomplishes the reduction of the reaction rate order by treating a reactant which is in large excess as constant.

Another approximation method resembling order of magnitude analysis is used to simplify the mathematical description of certain consecutive chemical reactions. Consecutive chemical reactions occur when the product of one reaction is the reactant in a subsequent reaction. The method referred to as the stationary state approximation (Harris, 1966), is applied when consecutive reactions occur at different rates, sometimes allowing a slowly changing time derivative term, describing the rate of formation or consumption of a chemical species, to be set equal to zero. As a result, an unsteady state equation is approximated by an equilibrium or steady state equation. Appropriately, the quasi-steady state method is known in the Russian literature as "the method of quasistationary concentrations" (Emanuel' and Knorre, 1973).

#### Ground Water-Lake Interactions

Ground water-lake interactions are poorly understood and are often overlooked in lake water and nutrient budgets (Winter, 1978). Some recent studies have used seepage meters to measure, in situ, the direction and magnitude of seepage flux through lake sediments. The chemical composition of samples taken from seepage meters may be effected by the environment created by these devices.

Karaskas and Anderson (1978) suggest that flow through conditions occur at Nepco Lake (Wisconsin) based on water table contours around the lake. High manganese concentrations down gradient of Nepco Lake were attributed to the dissolution of manganese in the lake sediments. A similar analysis was done by Loeb and Goldman (1979) to estimate the ground water contribution of Ward Valley to Lake Tahoe. Ground water flow during 1975 was found to be 16% of the Ward Creek inflow to Lake Tahoe. Ground water contributions of nitrates and soluble phosphorus were found to be 78% and 90%, respectively, of the Ward Creek loading.

Dilutional pumping was carried out in 1970 to improve the water quality of Snake Lake (Born et al., 1973). With ground water phosphorus concentrations of about 0.050 mg/l it was expected that the lake water phosphorus concentration (0.4 mg/l) would be diluted. Pumping increased ground water velocities from about 3.5 to 28  $\mu\text{m}/\text{sec}$ . The initial increases of chloride, nitrogen and phosphorus in the lake water were thought to be caused by flushing from lake sediments due to the high sediment interstitial velocities induced by pumping.

Cartwright et al. (1979) suggest that ground water movement into Lake Michigan explains the distribution of trace elements in bottom sediments. Trace elements decrease in concentration downward from the lake water--sediment interface. Results from piezometer measurements showed that the ground water flux was

from the aquifer into the lake. Hence, the ground water which has twice the mineral content and hardness as the lake water moves through the lake sediments and trace elements are precipitated out near the sediment--lake water interface.

### Seepage Meters

The direct measurement of aquifer discharge and recharge in lake environments can be made with seepage meters described by Lee (1977) and by Lock and John (1978). Lee (1977) describes a simple device consisting of a 55 gallon drum with one open end and an outlet on the other end for sample collection. Lock and John (1978) describe a seepage meter provided with a container to buffer the sampling vessel from outside water movement. Also, a gas/pressure release valve was mounted on the top of the meter to allow the measurement of gas which may be released from the sediments during the sampling period.

Theoretical models of ground water-lake interactions can be verified using seepage meters. Also chemical analysis of seepage water may provide information on the effect of sediment chemistry on the chemistry of ground water and vice versa.

Patterns of ground water flow through lake sediments were measured using seepage meters (Fellows and Brezonick, 1980; Connor and Belanger, 1981; Lock and John, 1978; Lee et al., 1980;

Brock et al., 1982). Maximum seepage velocities were found to occur near shore and to decrease exponentially with distance from the shore line.

Lee (1976) examined the reliability of seepage meters as ground water collection devices at Lake Sallie (Minnesota). Nutrient concentrations in seepage water became similar to nutrient concentrations in adjacent wells after more than 1,200 liters of water had passed through the seepage meters. Keel (1979) found that ground water nutrient contributions to lakes in the Upper Pelican River Watershed of Minnesota were significant. The average total phosphorus and total nitrogen concentration of seepage water ranged from 1.04-1.63 and 1.59-4.0 mg/l, respectively.

A theoretical model based on ground water contours around Perch Lake (Ontario) was confirmed using seepage meters and a tritium tracer (Frape and Patterson, 1981). The authors suggest that lake water mixing with sediments would be minimal in aquifer discharge areas which generally occur near shore and deeper mixing would be found in deep lake sediments. Accordingly, it was noted that a thick metal zone occurred in deep lake sediments, due to mixing with oxygen rich lake water, while a thin metal enriched zone was found in near shore sediments. Lee et al. (1980) suggest that sediment biology may affect the ground water chemistry as ground water enters a lake and that ground water may flush materials from lake sediments into the lake.



Brock et al. (1982) estimate ground water seepage into Lake Mendota using seepage meters. Because seepage water has a long residence time in the seepage meters, compared with the time it would take for the seepage water to undergo chemical modification, seepage meter samples were not considered reliable for estimating nutrient flux from ground water. Therefore, both dialysis chambers and pipeting were used to gather sediment pore water samples for chemical analysis.

Spacial variability in the chemical composition of sediment pore water was attributed to either differences in groundwater chemistry or the modification of seepage water by lake sediments as the seepage water moved into the lake. The average phosphorus concentration of seepage water was found to be 171.5  $\mu\text{g}/\text{l}$ . Seepage meter data show that ground water accounts for one third of the inflowing water to Lake Mendota. Chemical analysis of seepage water shows, however, that seepage represents only 12% of the phosphorus loading to the lake.

Comparisons of seepage water nutrient concentrations with that of wells in the vicinity indicates that seepage water is higher in phosphorus and ammonia than well water. The authors suggest that the phosphorus and ammonia in seepage water was derived from the lake itself and is, therefore, being recycled from the sediments aided by ground water discharge through the lake sediments.

## C H A P T E R I I I

## FIELD MEASUREMENTS AND DATA COLLECTION

Background

Lake Warner was created in the early 1800's by the construction of a dam on the Mill River (Hadley, Massachusetts). By-passes from the Amherst Wastewater Treatment Plant flowed into the Mill River, approximately 1.5 miles (2.4 (10)<sup>3</sup> m) upstream from Lake Warner, in the late 1960's and early 1970's, causing the lake to experience excessive algal blooms. Wastewater discharges were stopped in November, 1975, with the completion of the new Amherst Wastewater Treatment Plant. Hence, Lake Warner provided a good study area for assessing the effect of reducing nutrient loadings to a eutrophic lake.

The Lake Warner watershed was analyzed by Jubinville (1973). The total area drained by the Mill River was determined to be 32.1 square miles (8313.8 hectares) and the land usage in this watershed is shown in Table 2.

Table 2. Percentage of Land Usage in the Lake Warner Drainage Basin (after Bingham and Feng, 1980)

Land Use Type	Percentage
Agricultural (Dairy Farms)	20%
Agricultural (Crops)	20%
Non-Agricultural (Forest)	50%
Urban	10%

Snow and DiGiano (1976) determined the lake area to be 68 acres (27.5 hectares), the lake volume to be 352 acre-feet (4.35 (10)<sup>5</sup> m<sup>3</sup>) and the average depth to be 5.15 feet (1.58 m).

Lake Warner sediments were shown to reflect eutrophic lake characteristics (Snow and DiGiano, 1973) based on the high concentration of PO<sub>4</sub><sup>-3</sup>-P in the sediment which would be available for exchange with the lake water to support algal blooms. Snow and DiGiano (1976) estimated the average porosity of the upper five centimeters of the lake sediment to be 0.84. Also, high sediment solid phase and interstitial phosphorus concentrations were found in Lake Warner sediments. They measured sediment solid phase phosphorus concentrations of 0.9 to 1.6 mgP/g of dry sediment and 320 to 450 μg/l sediment interstitial total phosphorus concentrations. The lake water total phosphorus concentrations averaged about 90 μg/l in 1973 and 1974. Snow and DiGiano (1976) also found an extreme dissolved oxygen gradient in the lake water during the summer of 1973. Excessive algal blooms (including a blue-green variety) occurred throughout the summers of 1973 and 1974.

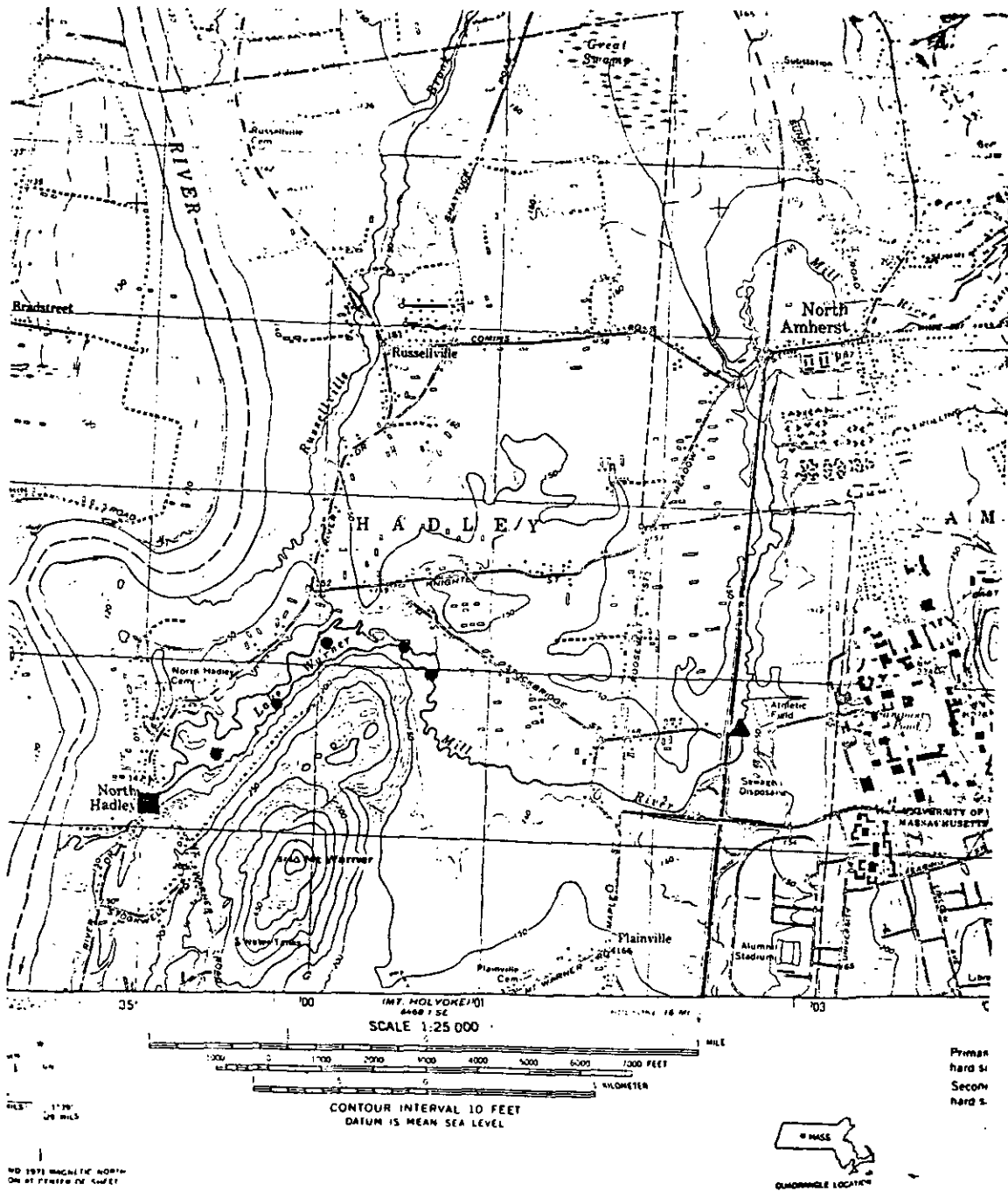
Bingham and Feng (1980) found that Lake Warner had recovered from its eutrophic state more quickly than had been predicted by the Snow and DiGiano (1976) lake recovery model. The average total phosphorus concentration of the lake water was found to be 50 μg/l in 1976. Algal blooms were of only very short duration during mid July of 1976. They found that sediment interstitial

total phosphorus concentrations ranged from 150 to 300  $\mu\text{g}/\text{l}$  and that the sediment solid phase phosphorus concentration was actually higher than that measured in 1973 by Snow and DiGiano (1976). This was attributed to more consistently oxic lake bottom waters which increased the phosphorus binding capacity of the lake sediments.

Visual observations during the summer of 1981 indicate that Lake Warner has indeed recovered from its former eutrophic state. During the low flow and high temperature period of July algal blooms were of only short duration. Filamentous blue-green species were not excessive but Watermeal (Wolffia brasiliensis) and Duckweed (Lemna minor) did cover much of the lake surface during portions of the summer of 1981.

#### Materials and Methods

To follow up the studies by Snow and DiGiano (1976) and Bingham and Feng (1980) the Mill River and Lake Warner were sampled for phosphorus concentration on July 31, 1981 and daily during a period from September 18, 1981 to October 4, 1981. Stream gauging and sampling was carried out on the Mill River (stations shown in figure 6) to estimate the phosphorus budget of Lake Warner and to model the release of phosphorus from the lake bottom. Also, measurements were made on the ground water component of Lake Warner hydrology using seepage meters. Measurements taken during the dry months of July and August are compared to



Symbol

Sampling Location



Influent sampling location for mass balance shown in Figure 11.



Effluent sampling location used in Figure 11.



Sampling locations used in Figures 9 and 10.

Figure 6. Location map and water sampling sites.

measurements taken during October and November of 1981 (see figure 13).

Water samples were analyzed for total and orthophosphate phosphorus using the Heteropoly Blue-Ascorbic acid Spectrophotometric method (Strickland and Parsons, 1972). Samples from Lake Warner were taken at one-third meter below the surface and from one meter above the bottom. Bottom samples were taken with a Kemmerer sampler. Samples were transported to the laboratory in 500 milliliter acid washed bottles. All glassware used in the phosphorus analysis were acid washed.

Water samples were analyzed for both orthophosphate and total phosphorus on the day that they were collected. Samples analyzed for orthophosphate were filtered through 0.45  $\mu$ m filters, using a millipore apparatus, prior to the addition of the composite reagent solution. Total phosphorus samples were autoclaved after the addition of 0.4 ml of 11N  $H_2SO_4$  and 0.2 grams of potassium persulfate. These samples were allowed to cool to room temperature before pH adjustment and the addition of the composite reagent solution. The absorbance of the prepared samples was measured on a Bausch and Lomb Spectronic 70 spectrophotometer. A ten centimeter cell was used. The calibration curves of total and orthophosphate phosphorus concentration vs. absorbance appear in Appendix A.

Stream gauging was done on the inlet and outlet streams of Lake Warner using the U.S.G.S. mid-section method (Buchanan and Somers, 1969). Stream velocities were measured at 0.6 of the

stream depth with a Gurley pygmy current meter. A stage-discharge curve was developed using stream flow measurements and a staff gauge to measure stream stage. These curves appear in Appendix A. Stream flow on the sampling dates was estimated by measuring the stream stage and finding the flow from the stage-discharge curve.

Seepage through Lake Warner sediments was measured during July, August and November 1981 using seepage meters described by Lee (1977). The basic shell of a seepage meter consists of a 208.2 liter (55 gallon) steel drum cut at 25 centimeters (9.8 in.) from the end (see Fig. 7). The insides of the seepage meters were painted with epoxy resin to prevent corrosion. The seepage meters were installed in the lake bottom by turning them slowly into the lake sediments until approximately ten centimeters of the meter extended above the sediment-lake water interface. Tubes for sample collection were then inserted in the the bung holes which had been cut into the tops of the meter and Ziplock heavy duty freezer bags were attached to them. Five hundred milliliters (0.132 gal.) of distilled water was placed in each bag before it was attached to a seepage meter.

Meters 1 through 5 were installed on the southeast side of Lake Warner and meters 1A through 5A were installed on the opposite side of the lake as can be seen from Figure 8. The meters were placed in the lake sediments 1.5 to 3 meters (five to ten feet) from shore in about 0.6 meters (two feet) of water. Sample bags were changed from a twelve foot aluminum boat.

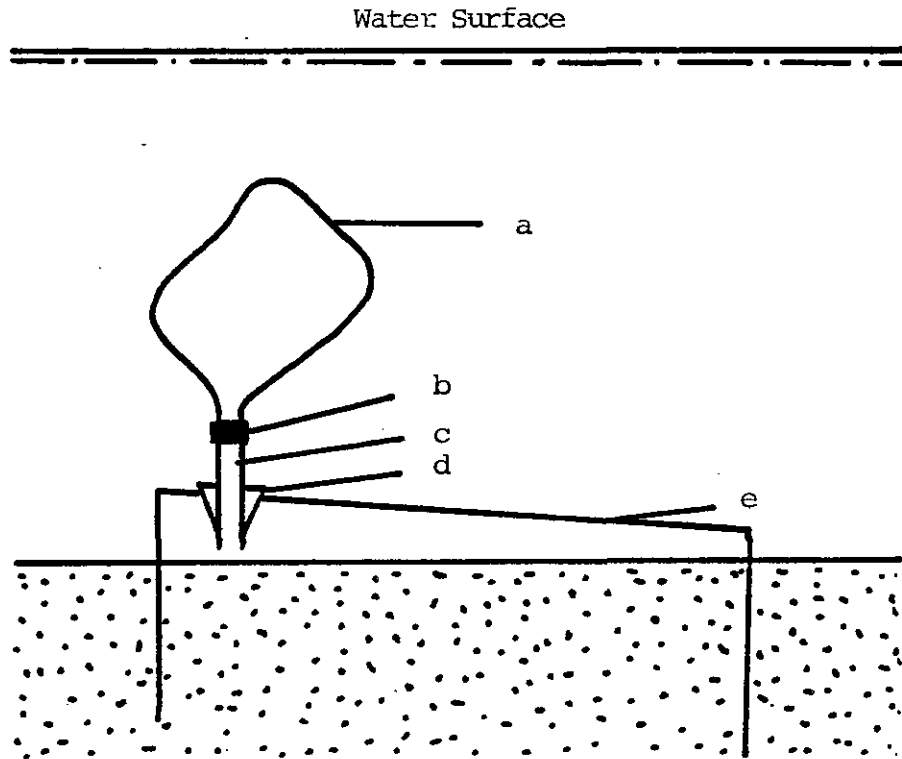


Figure 7. Section view of seepage meter. a - Ziplock heavy duty freezer bag; b - rubber band; c - 1/4" ID polyethylene tube; d - No. 10 rubber stopper; e - end section of steel drum.



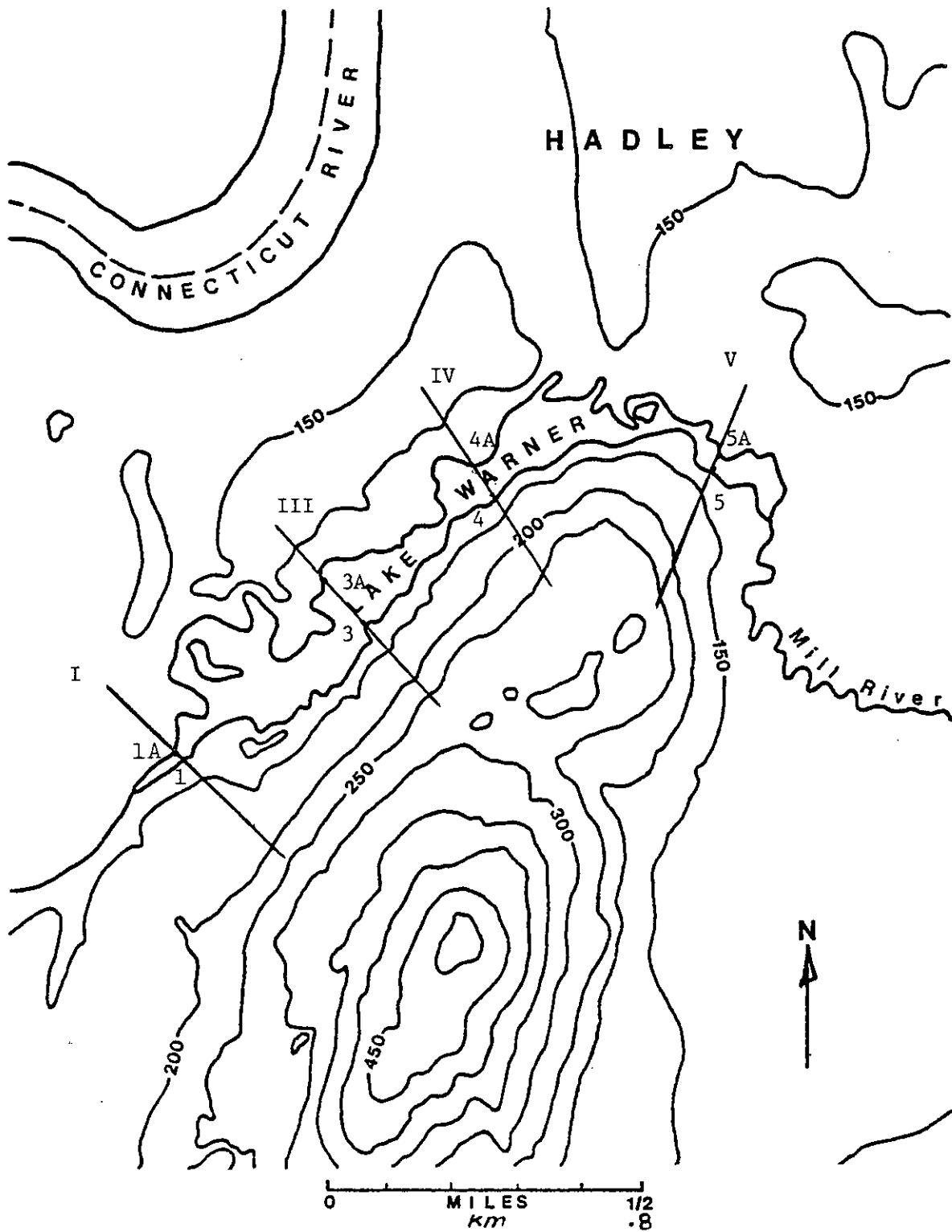


Figure 8. Location map showing seepage meter station and the locations of cross sections.

To estimate seepage rates the net changes in water volume in the sample bags were measured. Measurements were done by weight or volumetrically. Thus if, during the sampling period, there was a net loss of water from the sample bag (each bag was installed with a content of 500 ml of distilled water) then the ground water aquifer was being recharged through the lake sediments. If after the sampling period the sample bag contained greater than the original 500 ml of water then the ground water aquifer was discharging to the lake. These measurements represent a macro-seepage velocity and should not be confused with average interstitial velocities which can be obtained by dividing the seepage rate by the porosity of the sediment. The seepage rates, herein, were estimated by measuring the volume of seepage water (described above) and the duration of the sampling period. Given the area of sediment that was enclosed by the seepage meter the seepage rate can be calculated:

$$u = V/tA \quad 3.1$$

With volume (V) in liters and time (t) in hours and Area (A) of  $0.255 \text{ m}^2$  the seepage rate (u) in  $\mu\text{m}/\text{sec}$  can be expressed as:

$$u (\mu\text{m}/\text{sec}) = 1.089 (V/t) \quad 3.2$$

### Discussion

Low flow conditions existed throughout the sampling period in this study. Lake water phosphorus concentration data from July 31, 1981 (Fig. 9) shows a typical dry weather phosphorus profile for surface water and bottom water along an east-west transect through Lake Warner. Data from the intensive sampling period of September 18, 1981, to October 4, 1981 are averaged (Fig. 10) and show a phosphorus profile similar to the wet weather data of Bingham and Feng (1980).

Figure 11 shows the Lake Warner phosphorus budget as determined by stream flow measurements and phosphorus analyses. A net export of phosphorus occurred on the July 31, 1981 sampling date and a net retention of phosphorus occurred during the intensive sampling period (September 18, 1981 to October 4, 1981). Lake sediments released phosphorus during periods of high water temperature and low flow conditions while sediment retention of phosphorus occurs at lower water temperatures and reduced biological activity (Bingham and Feng, 1980). Hence, for Lake Warner, sediment release of phosphorus is seasonal and of short duration, thus only temporarily affecting the nutrient balance of the lake.

The hydrogeologic setting of Lake Warner was examined by constructing cross sections through the lake using a Mount Toby U.S.G.S. quadrangle map and a contour map of the bedrock surface (Londquist, 1974). These cross sections of the bedrock and

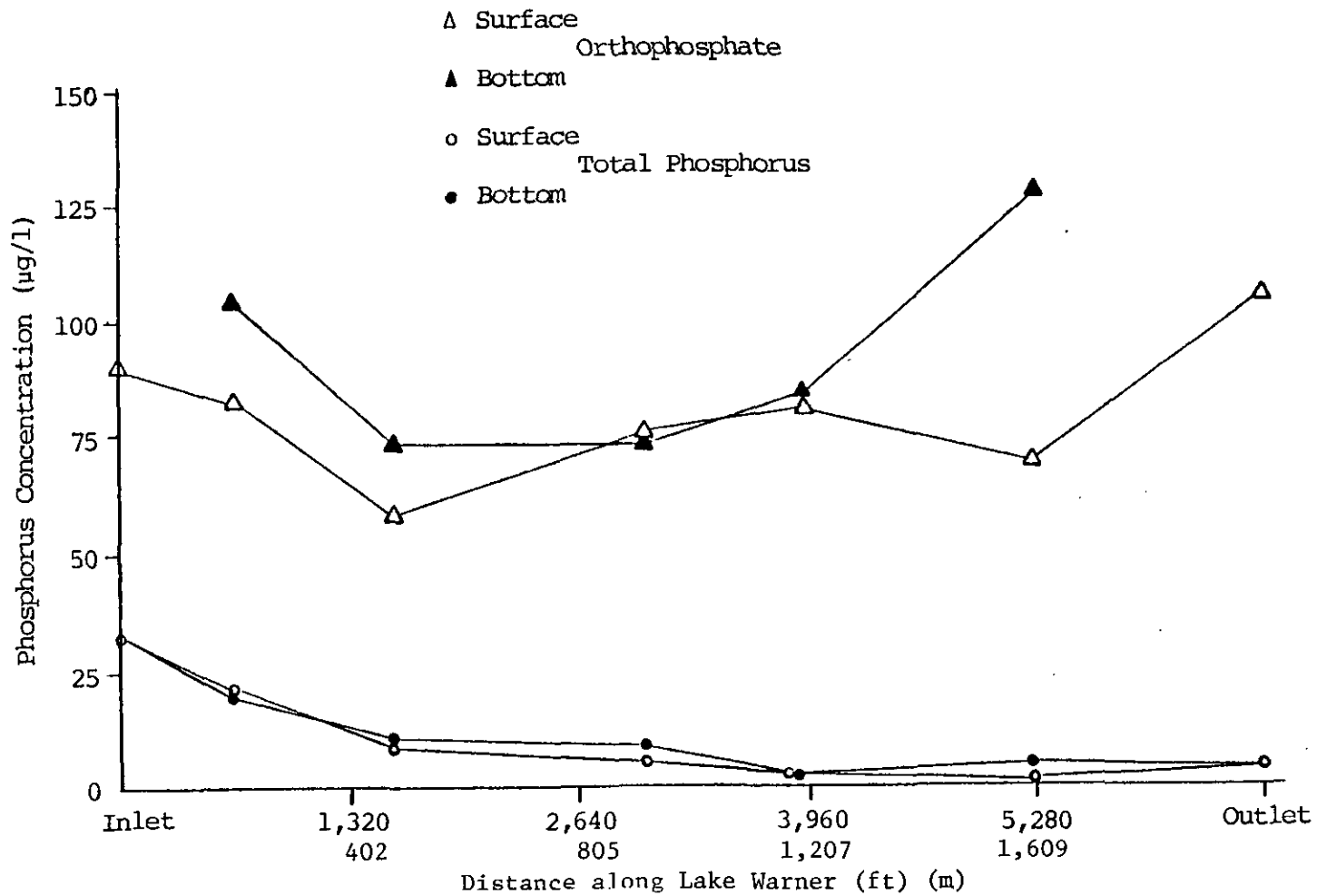


Figure 9. Lake-water phosphorus concentrations for sampling date 7/31/81.

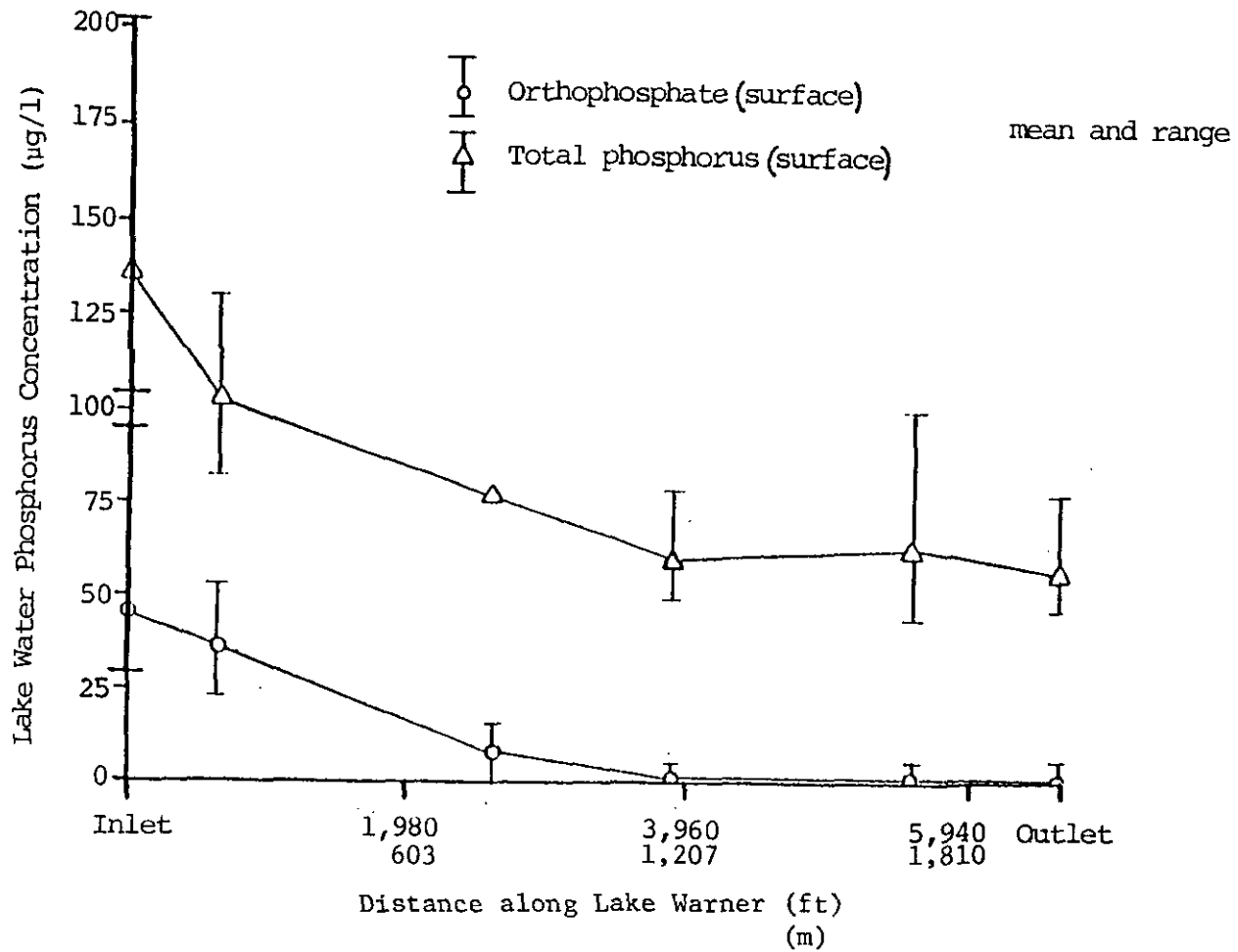


Figure 10. Average lake water phosphorus concentrations for sampling period 9/18/81-10/4/81.

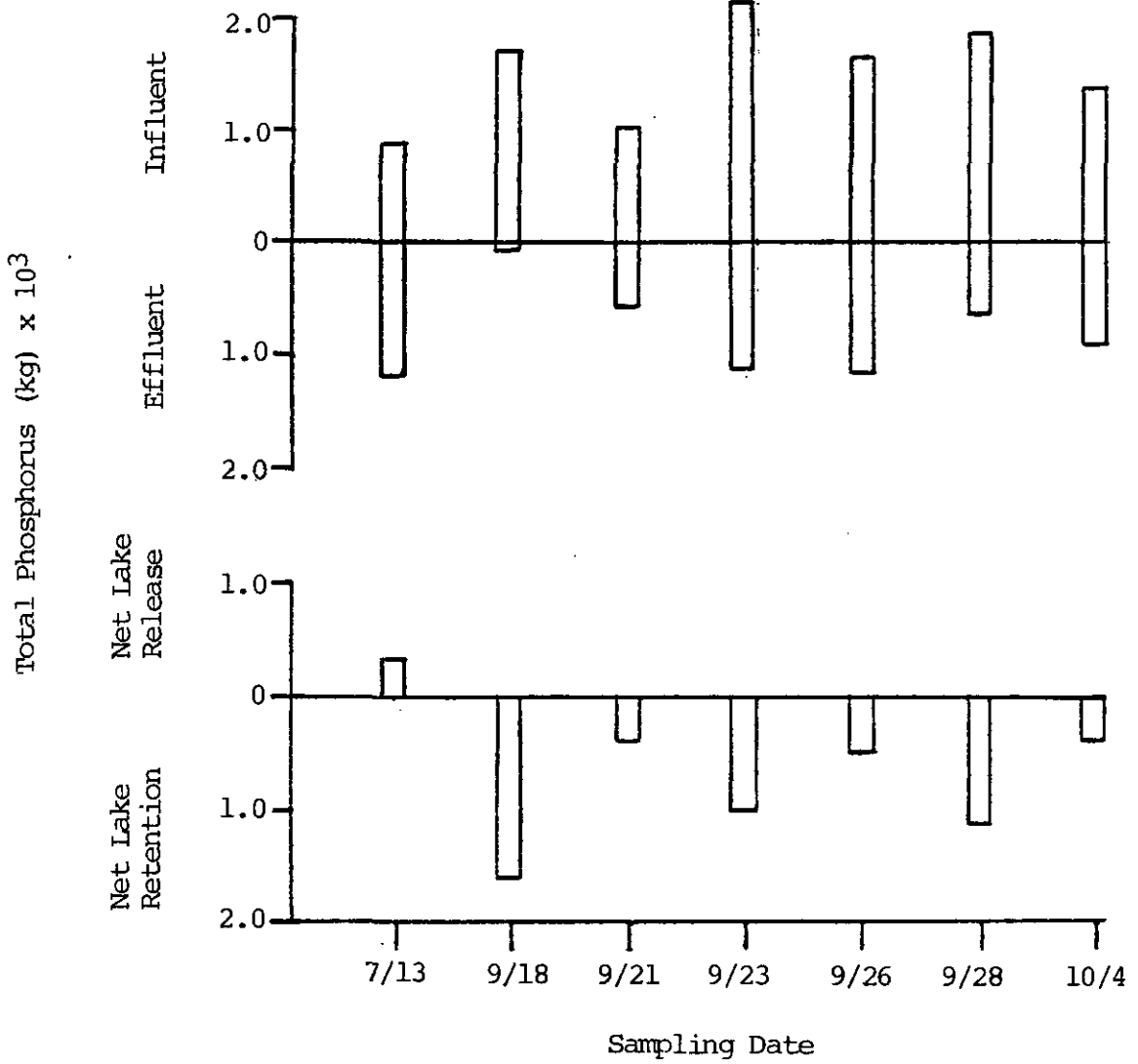


Figure 11. Mass balance on lake water total phosphorus.

overburden elevations appear in Figure 12 and suggest that ground water may discharge into Lake Warner from the southeast and be recharged on the northeast side of the lake.

Figure 13 shows that ground water enters the lake (hence, positive velocities) at meter locations 1, 2, 3, 5 and 5A, and that lake water flows out of the lake at seepage meter locations 1A, 2A, 3A, 4 and 4A. Hence, a flow through condition exists between cross sections I and III. A flow out of the lake occurs at cross section IV and a flow into the lake occurs at cross section V.

Seepage velocity measurements taken at Lake Warner in November 1981 were generally double those taken during July and August 1981. Reversals in seepage flow that occurred at meter locations 1A, 2A, 3A, 4 and 4A were probably due to ground water mounding caused by the precipitation that occurred in October and November 1981 and the reduction of evapotranspiration by vegetation near the lake.

The importance of ground water to Lake Warner's water budget can be examined by using a seepage rate estimated by field measurements ( $u_{\max} \sim 10^{-2} \mu\text{m}/\text{sec}$ ) and assuming that this seepage rate occurs throughout the lake bottom ( $\text{Area}(A) = 2.572(10)^5 \text{m}^2$ ).

Then one estimates: Maximum Seepage flow rate =  $Au \sim 222 \text{m}^3/\text{day}$

Average Mill River flow rate  $\sim 48902 \text{m}^3/\text{day}$

Comparing the seepage flow rate ( $222 \text{m}^3/\text{day}$ ) with the Mill River flow rate ( $48902 \text{m}^3/\text{day}$ ) makes it clear that seepage flow can be neglected in the water budget of Lake Warner.

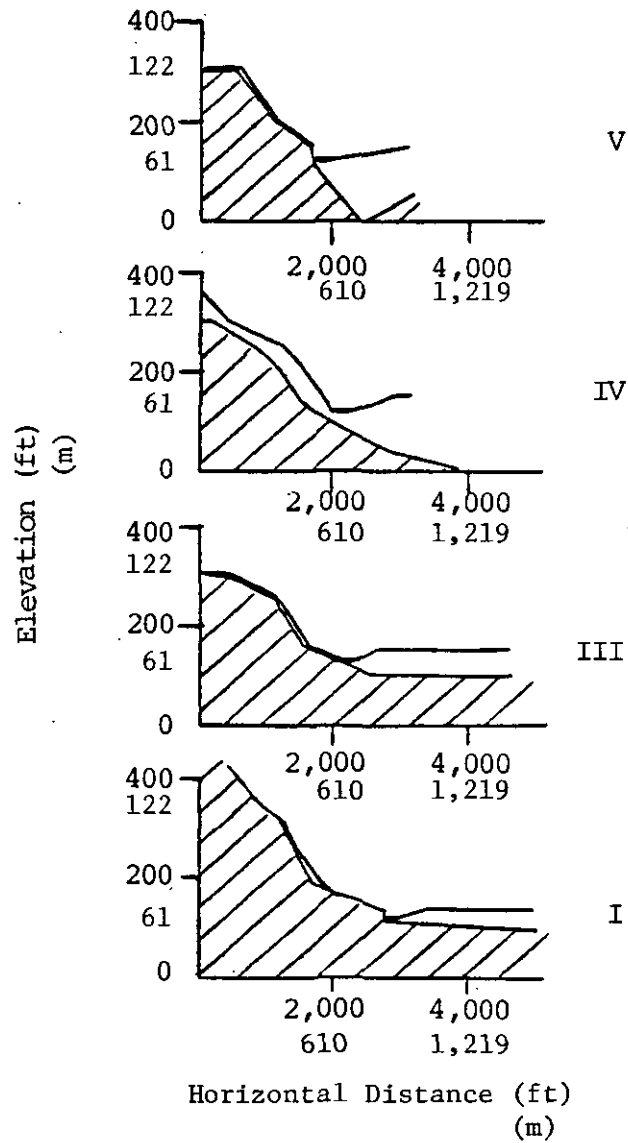


Figure 12. Cross sections through Lake Warner showing the bedrock overburden elevations. The locations of cross sections are shown in Figure 8.



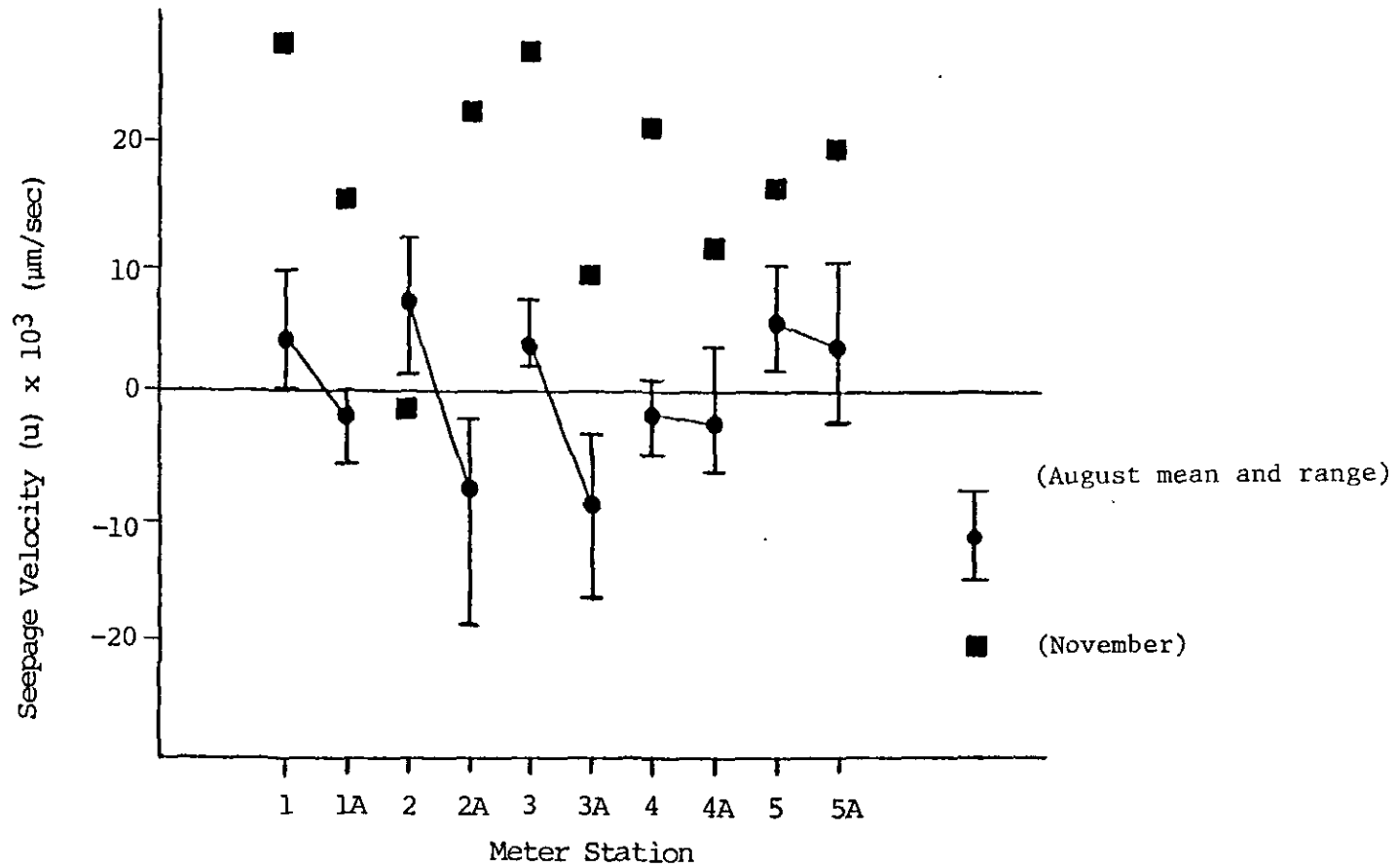


Figure 13. Seepage velocities. Negative seepage velocities mean seepage out of the lake. The station locations are shown in Figure 8.

The significance of ground water discharge through lake sediments as a nutrient transport mechanism can be examined by comparing seepage flux with the phosphorus loading from the Mill River. The average flow rate, determined by Snow and DiGiano (1976), ( $48902.4 \text{ m}^3/\text{day}$ ) and the average phosphorus concentration of  $50 \mu\text{g}/\text{l}$  (as determined by Bingham and Feng, 1980) were used to estimate Mill River phosphorus loading. Average sediment interstitial phosphorus concentrations of  $176 \mu\text{g}/\text{l}$  as measured by Bingham and Feng (1980) were used. The maximum value for seepage rates, determined in this study to be about  $10^{-2} \mu\text{m}/\text{sec}$  through Lake Warner sediments were assumed to occur uniformly through the lake bottom. This is also a high estimate as seepage was found to recharge the ground water aquifer over part of the lake bottom during this study. One can then estimate the relative phosphorus loadings from the Mill River and from seepage as:

$$\text{Average Mill River Loading} \sim 2.4(10)^6 \text{ mgP/day}$$

$$\text{Maximum Seepage Loading} \sim 3.9(10)^4 \text{ mgP/day}$$

As can be seen from the above calculation, the maximum expected seepage loading of phosphorus to Lake Warner is very small ( $3.9(10)^4 \text{ mgP/day}$ ) compared with the average Mill River phosphorus loading ( $2.4(10)^6 \text{ mgP/day}$ ). Hence, phosphorus loading due to ground water discharge to the lake can be neglected in the construction of a phosphorus budget for Lake Warner.

C H A P T E R   I V  
L A K E   R E C O V E R Y   M O D E L

The lake recovery model developed by Snow and DiGiano (1976) consists of three control volumes (on lake water phosphorus, and on lake sediment interstitial and sediment solid phase phosphorus) which are shown in Figure 14. Control volume I shows the mechanisms by which phosphorus is transported through the lake water. Snow and DiGiano (1976) showed that Lake Warner more closely approximated a Continuous Flow Stirred Tank Reactor (CFSTR) than a Plug Flow Reactor. Hence, CFSTR assumptions were used in their lake recovery model. These assumptions are:

- i) The lake is completely mixed.
- ii) The outflow phosphorus concentration is the lake water phosphorus concentration.
- iii) The Mill River flow rate ( $Q$ ) is assumed to be constant on an annual average time scale. The change in lake water storage can therefore be neglected.
- iv) The Mill River influent phosphorus concentration  $P_0$  is constant.
- v) The mass transfer rate ( $K_1$ ) and the reaction rates ( $K_2$ ) and ( $K_3$ ) is constant.

Mass balance equations were developed for the control volumes shown in Figure 14 under the assumption that both total phosphorus and water are conservative substances. The following

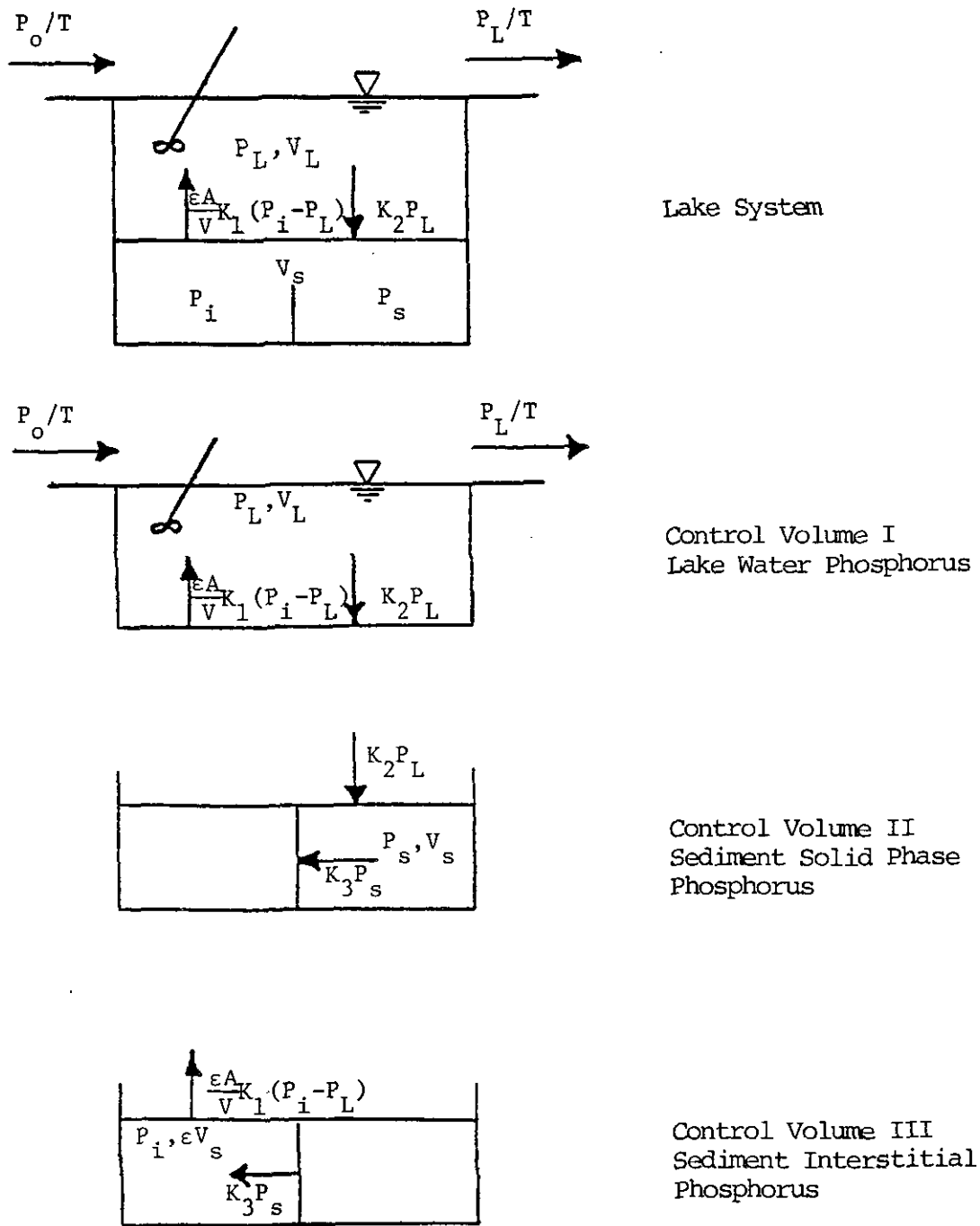


Figure 14. Control volumes used in the lake phosphorus budget model.

discussion will use the term "phosphorus" as synonymous with total phosphorus which is the conservative substance being modeled in the lake system.

#### Release Rate ( $K_1$ )

The release of phosphorus from lake bottoms is governed by physical, chemical and biological processes. The rate limiting step for phosphorus release from the sediments could emanate from resistance due to pore diffusion rates through the interstitial water or resistance at the interface of the sediment deposit and the lake water (DiGiano, 1971). Because concentrations of sediment phosphorus in eutrophic lakes is high, it can be assumed that interfacial resistance entirely controls the rate of mass transport. The dominance of the interfacial resistance effect over pore diffusion rates in deeper sediments can be defended by examining the magnitude of the sediment solid phase phosphorus concentration in the upper sediment layers.

The concentration of exchangeable phosphate in Lake Warner sediments is on the order of  $10^5 \mu\text{g}/\text{l}$  (Snow and DiGiano, 1976). With an average lake water depth of about 1.7 meters, 0.1 meters of sediment (0.1 meters is the exchange depth used by Snow and DiGiano, 1976) could replace a lake water phosphorus concentration of  $100 \mu\text{g}/\text{l}$  about 60 times. Therefore, with an average lake

water residence time of about 10 days the sediments alone contain a phosphorus source capable of keeping the lake water at 100  $\mu\text{g}/\text{l}$  for 600 days.

DiGiano (1971) suggests a simple model of interfacial resistance to describe the rate of mass transport in deposit-water systems. In the case of lake bottom phosphorus the release is proportional to the difference in the sediment interstitial phosphorus concentration ( $P_i$ ) and the phosphorus concentration in the overlying water ( $P_L$ ). The release of phosphorus directly from the solid phase to the lake water is negligible because of the small area of sediment solid phase phosphorus in direct contact with the lake water compared with the area in contact with interstitial water. The exchange depth, estimated by Snow and DiGiano (1976) to be 0.1 meters, multiplied by the lake area is the volume of lake bottom allowed to release phosphorus to the lake water. The volume of sediment solid phase phosphorus ( $P_s$ ) is represented by  $V_s$  (rather than  $(1 - \epsilon)V_s$ ) and the sediment interstitial phosphorus ( $P_i$ ) as  $\epsilon V_s$ . This is appropriate because  $P_s$  is three orders of magnitude larger than  $P_i$ . Snow and DiGiano (1976) carried out an in situ caisson study at Lake Warner to determine the value of the mass transfer coefficient ( $K_1$ ) which has a magnitude of about 0.1 meters/day. Hence, the lake bottom phosphorus release can be described as:

$$R_{\text{rel}} = \frac{\epsilon A}{V_s} K_1 (P_i - P_L) \quad 4.1$$

Sedimentation Rate ( $K_2$ )

The removal of phosphorus from the lake water to the sediment can be modeled by assuming a first order reaction rate ( $K_2$ ) which describes all physical, chemical and biological processes in the lake (Snow and DiGiano, 1976).  $K_2$  has a magnitude of about 0.1/day. The rate of removal of phosphorus from the lake water (assumed to be proportional to the lake water phosphorus concentration) to the sediment can be described as:

$$R_{\text{sed}} = K_2 P_L \quad 4.2$$

Conversion Rate ( $K_3$ )

The modification of the Snow and DiGiano (1976) model by Bingham and Feng (1980) to include the reaction rate  $K_3$  required the introduction of a mass balance equation on sediment interstitial phosphorus.

The reaction rate ( $K_3$ ) describes the conversion of sediment solid phase phosphorus ( $P_s$ ) to sediment interstitial phosphorus ( $P_i$ ). It is a composite parameter which describes all physical, chemical and biological processes in the sediment.  $K_3$  is assumed to be constant although seasonal fluctuations in  $K_3$  values might be expected due to variations in Mill River water quality and flow rate which affects lake water temperature, dissolved oxygen, biological activity and sediment redox potential.

The rate of conversion from sediment solid phase to interstitial phosphorus is assumed to be proportional to the sediment solid phase phosphorus concentration.

$$\begin{array}{l} \text{Rate of conversion from solid} \\ \text{to interstitial phosphorus in} \\ \text{sediment.} \end{array} = K_3 P_s \quad 4.3$$

### Mass Balance Equations

The basic equation for the conservation of a reactive material is:

$$\begin{array}{l} \text{Rate of} \\ \text{Mass Input} \end{array} - \begin{array}{l} \text{Rate of} \\ \text{Mass Output} \end{array} + \begin{array}{l} \text{Rates of} \\ \text{Reaction} \end{array} = \begin{array}{l} \text{Rate of} \\ \text{Accumulation} \end{array} \quad 4.4$$

The terms of the above mass balance equation for the lake water phosphorus control volume (control volume I in Fig. 14) become:

$$\text{Mass Input Rate} = QP_o + \epsilon AK_1 (P_i - P_L) \quad 4.5$$

$$\text{Mass Output Rate} = QP_L \quad 4.6$$

$$\text{Rate of Reaction} = V_L K_2 P_L \quad 4.7$$

$$\begin{array}{l} \text{Rate of Mass} \\ \text{Accumulation} \end{array} = V_L \frac{\Delta P_L}{\Delta t} \quad 4.8$$

Substituting equations (4.5, 6, 7 and 8) into equation (4.4) and letting the deltas go to zero yields:



$$V_L \frac{dP_L}{dt} = QP_0 - QP_L - V_L K_2 P_L + \epsilon AK_1 (P_i - P_L) \quad 4.9$$

The terms for the mass balance equation for the sediment solid phase phosphorus control volume (control volume II in Fig. 14) become:

$$\text{Mass input rate} = V_L K_2 P_L \quad 4.10$$

$$\text{Mass output rate} = V_s K_3 P_s \quad 4.11$$

$$\begin{aligned} \text{Rate of Mass} &= V_s \frac{\Delta P_s}{\Delta t} \\ \text{Accumulation} & \end{aligned} \quad 4.12$$

Substituting equations (4.10, 11 and 12) into equation (4.4) and letting the deltas go to zero yields:

$$V_s \frac{dP_s}{dt} = V_L K_2 P_L - V_s K_3 P_s \quad 4.13$$

The terms for the mass balance equation for the sediment interstitial phosphorus control volume (control volume III in Fig. 14) become:

$$\text{Mass Input Rate} = V_s K_3 P_s \quad 4.14$$

$$\text{Mass Output Rate} = \epsilon AK_1 (P_i - P_L) \quad 4.15$$

$$\begin{aligned} \text{Rate of Mass} &= \epsilon V_s \frac{\Delta P_i}{\Delta t} \\ \text{Accumulation} & \end{aligned} \quad 4.16$$

Substituting equations (4.14, 15 and 16) into equation (4.4) and letting the deltas go to zero yields:

$$V_s \frac{dP_i}{dt} = V_s K_3 P_s - \epsilon AK_1 (P_i - P_L) \quad 4.17$$

Hence, the modified Snow and Digiano (1976) model as presented by Bingham and Feng (1980) is:

Sediment Solid Phase Phosphorus

$$V_s \frac{dP_s}{dt} = -V_s K_3 P_s + V_L K_2 P_L \quad 4.13$$

Sediment Interstitial Phosphorus

$$V_s \frac{dP_i}{dt} = V_s K_3 P_s - \epsilon AK_1 (P_i - P_L) \quad 4.17$$

Lake Water Phosphorus

$$V_L \frac{dP_L}{dt} = QP_O - QP_L + \epsilon AK_1 (P_i - P_L) - V_L K_2 P_L \quad 4.9$$

The above equations can be rewritten in a form which is more convenient for solution by grouping the coefficients:

Sediment Solid Phase Phosphorus

$$\frac{dP_s}{dt} = X_1 P_s + X_2 P_L \quad 4.18$$

Sediment Interstitial Phosphorus

$$\frac{dP_i}{dt} = X_3 P_s + X_4 P_i + X_5 P_L \quad 4.19$$

Lake Water Phosphorus

$$\frac{dP_L}{dt} = X_6 P_i + X_7 P_L + X_8 \quad 4.20$$

where:

$$\begin{aligned} X_1 &= -K_3 \\ X_2 &= K_2 V_L / V_S \\ X_3 &= K_3 / \epsilon \\ X_4 &= -AK_1 / V_S \\ X_5 &= AK_1 / V_S \\ X_6 &= \epsilon AK_1 / V_L \\ X_7 &= -(Q + \epsilon AK_1 + K_2 V_L) / V_L \\ X_8 &= P_0 Q / V_L \end{aligned}$$

This system of ordinary, first order, linear differential equations with constant coefficients (equations 4.18, 19, and 20) can be solved numerically (Bingham and Feng, 1980) or analytically. Analytical methods of solving these equations were explored including the Laplace transform method, the power series method, the trial function method and approximation methods. The Laplace transform method was not chosen because the inverse transform appeared formidable. The power series method was rejected after calculations using a digital computer showed the series did not approach convergence after evaluating one hundred terms.

The trial function method was successful in leading to the solution of equations (4.18, 19 and 20). Application of approximation techniques allowed these equations to be simplified sufficiently so that they could be solved readily. Both the trial function method and the approximation techniques are discussed more fully in the following sections.

#### Trial Function Method

The trial function method (Stockton, personal communication) is useful for solving a small number of simultaneous ordinary first order linear differential equations with constant coefficients. The number of algebraic equations to be manipulated to determine unknown coefficients grows exponentially with the number of simultaneous differential equations to be solved so that three simultaneous differential equations may be a practical upper limit for the method. The lake model requires the simultaneous solution of 12 algebraic equations. The non-homogeneous differential equations in the lake model were converted to homogeneous equations to simplify algebraic manipulations. Details of the trial function method appear in Appendix B. The equations for the lake water ( $P_L(t)$ ), sediment interstitial ( $P_i(t)$ ) and sediment solid phase ( $P_s(t)$ ) phosphorus concentration as determined by the trial function method are:

$$\begin{aligned}
P_L(t) = & P_{L\infty} + (P_{L0} - P_{L\infty}) \left[ \frac{E(BF-CE)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} e^{\alpha t} + \right. \\
& \left. \frac{F(CD-AF)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} e^{\beta t} + \right. \\
& \left. \frac{D(AE-DB)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} e^{\gamma t} \right] \\
& + (P_{i0} - P_{i\infty}) \left[ \frac{E(E-F)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} e^{\alpha t} + \right. \\
& \left. \frac{F(F-D)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} e^{\beta t} + \right. \\
& \left. \frac{D(D-E)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} e^{\gamma t} \right] \\
& - (P_{s0} - P_{s\infty}) \left[ \frac{E(B-C)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} e^{\alpha t} + \right. \\
& \left. \frac{F(C-A)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} e^{\beta t} + \right. \\
& \left. \frac{D(A-B)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} e^{\gamma t} \right] \quad . \quad 4.21
\end{aligned}$$

To use equation (4.21) substitute the values for the initial and steady state conditions and make the following substitutions:

$$A = [(\alpha - X_4)(\alpha - X_7) - X_5 X_6] / X_3 X_6 \quad 4.21.1$$

$$B = [(\beta - X_4)(\beta - X_7) - X_5 X_6] / X_3 X_6 \quad 4.21.2$$

$$C = [(\gamma - X_4)(\gamma - X_7) - X_5 X_6] / X_3 X_6 \quad 4.21.3$$

$$D = [X_2 X_3 + X_5 (\alpha - X_4)] / (\alpha - X_4) (\alpha - X_1) \quad 4.21.4$$

$$E = [X_2 X_3 + X_5 (\beta - X_4)] / (\beta - X_4) (\beta - X_1) \quad 4.21.5$$

$$F = [X_2 X_3 + X_5 (\gamma - X_4)] / (\gamma - X_4) (\gamma - X_1) \quad 4.21.6$$

$$\begin{aligned}
P_i(t) = & P_{i\infty} + (P_{i0} - P_{i\infty}) \left[ \left[ \frac{E(BF-CE)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(CD-AF)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(AE-DB)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \\
& + (P_{s0} - P_{s\infty}) \left[ \left[ \frac{E(E-F)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(F-D)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(D-E)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \\
& - (P_{L0} - P_{L\infty}) \left[ \left[ \frac{E(B-C)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(C-A)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(A-B)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \quad 4.22
\end{aligned}$$

To use equation (4.22) substitute the values for the initial and steady state conditions and make the following substitutions:

$$A = X_2 X_6 / (\alpha - X_1)(\alpha - X_7) \quad 4.22.1$$

$$B = X_2 X_6 / (\phi - X_1)(\phi - X_7) \quad 4.22.2$$

$$C = X_2 X_6 / (\gamma - X_1)(\gamma - X_7) \quad 4.22.3$$

$$D = [(\alpha - X_1)(\alpha - X_4)(\alpha - X_7) - X_2 X_3 X_6] / X_5 (\alpha - X_1)(\alpha - X_7) \quad 4.22.4$$

$$E = [(\phi - X_1)(\phi - X_4)(\phi - X_7) - X_2 X_3 X_6] / X_5 (\phi - X_1)(\phi - X_7) \quad 4.22.5$$

$$F = [(\gamma - X_1)(\gamma - X_4)(\gamma - X_7) - X_2 X_3 X_6] / X_5 (\gamma - X_1)(\gamma - X_7) \quad 4.22.6$$

$$\begin{aligned}
P_s(t) = & P_{s\infty} + (P_{s0} - P_{s\infty}) \left[ \left[ \frac{E(BF-CE)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(CD-AF)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(AE-DB)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \\
& + (P_{L0} - P_{L\infty}) \left[ \left[ \frac{E(E-F)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(F-D)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(D-E)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \\
& - (P_{i0} - P_{i\infty}) \left[ \left[ \frac{E(B-C)}{[(E-D)(BF-CE) - (E-F)(BD-AE)]} \right] e^{\alpha t} + \right. \\
& \left[ \frac{F(C-A)}{[(F-E)(CD-AF) - (F-D)(CE-BF)]} \right] e^{\phi t} + \\
& \left. \left[ \frac{D(A-B)}{[(D-F)(AE-BD) - (D-E)(AF-CD)]} \right] e^{\gamma t} \right] \quad 4.23
\end{aligned}$$

To use equation (4.23) substitute the values for the initial and steady state conditions and make the following substitutions:

$$A = (\alpha - X_1) \quad 4.23.1$$

$$B = (\phi - X_1) \quad 4.23.2$$

$$C = (\gamma - X_1) \quad 4.23.3$$

$$D = X_3 / [\alpha - X_4 - X_6 X_7 / (\alpha - X_7)] \quad 4.23.4$$

$$E = X_3 / [\phi - X_4 - X_6 X_7 / (\phi - X_7)] \quad 4.23.5$$

$$F = X_3 / [\gamma - X_4 - X_6 X_7 / (\gamma - X_7)] \quad 4.23.6$$

In the above equations  $\alpha$ ,  $\varrho$  and  $\gamma$  are the roots ( $r_1$ ,  $r_2$  and  $r_3$ ) of the following equation.

$$r^3 - (X_1 + X_4 + X_7)r^2 + (X_1X_4 + X_1X_7 + X_4X_7 - X_4X_6)r + X_1X_4X_6 - X_1X_4X_7 - X_2X_3X_6 = 0 \quad 4.24$$

and

$$\begin{aligned} P_{L\infty} &= P_0 \\ P_{i\infty} &= P_0 X_2 / X_5 \epsilon + P_0 \\ P_{s\infty} &= -P_0 X_2 / X_1 \end{aligned}$$

These equations appear formidable but can be programmed on a TI-59 programmable calculator. The program coding for the equations (4.21, 22 and 23) appears in Appendix C. It is suggested that terms that contain  $\varrho t$  and  $\gamma t$  in the exponent position be neglected in applications of the equations.  $\varrho$  and  $\gamma$  are very large ( $\varrho = [10^{-1}]$  and  $\gamma = [1]$ , respectively) compared to  $\alpha$  ( $\alpha = [10^{-3}]$ ). Hence as  $t$  grows large ( $t > 100$  days) terms with large negative rate constants ( $\varrho$  and  $\gamma$ ) in the exponent go to zero quickly and can be neglected.

#### Approximation Techniques

Short term predictions. Over short time steps the value of the interstitial phosphorus concentration can be considered constant (DiGiano and Snow, 1976). The value of  $P_i$  in equation



(4.9) is large relative to the other terms (see order of magnitude analysis in Appendix B) and can therefore be considered constant. This is similar to pseudo-order assumptions which allow species in excess in a chemical reaction to be considered constant to simplify the expression for the differential rate equation. In the initial period ( $t < 3T$ ) after phosphorus loading is reduced, dilution plays an important role in lake recovery. Hence, holding  $P_i$  constant and integrating equation (4.9) to give equation (4.25) yields a short term model that is most influenced by the dilution process but allows for phosphorus release from the lake bottom.

$$P_L(t) = -(X_8 + X_6 P_{i0})/X_7 + [(X_8 + X_6 P_{i0})/X_7 + P_{L0}]e^{X_7 t} \quad 4.25$$

Figure 15 compares predictions of the short term model (equation 4.25) with predictions of the trial function solution (equation 4.21). Figure 16 compares the same equations on an expanded time scale. It is evident that the short term predictions are accurate for only two or three hydraulic residence times (the average annual hydraulic residence time for Lake Warner is 10 days).

After the initial rapid reduction of the lake phosphorus concentration due to dilution, phosphorus release from the lake bottom may control lake recovery.

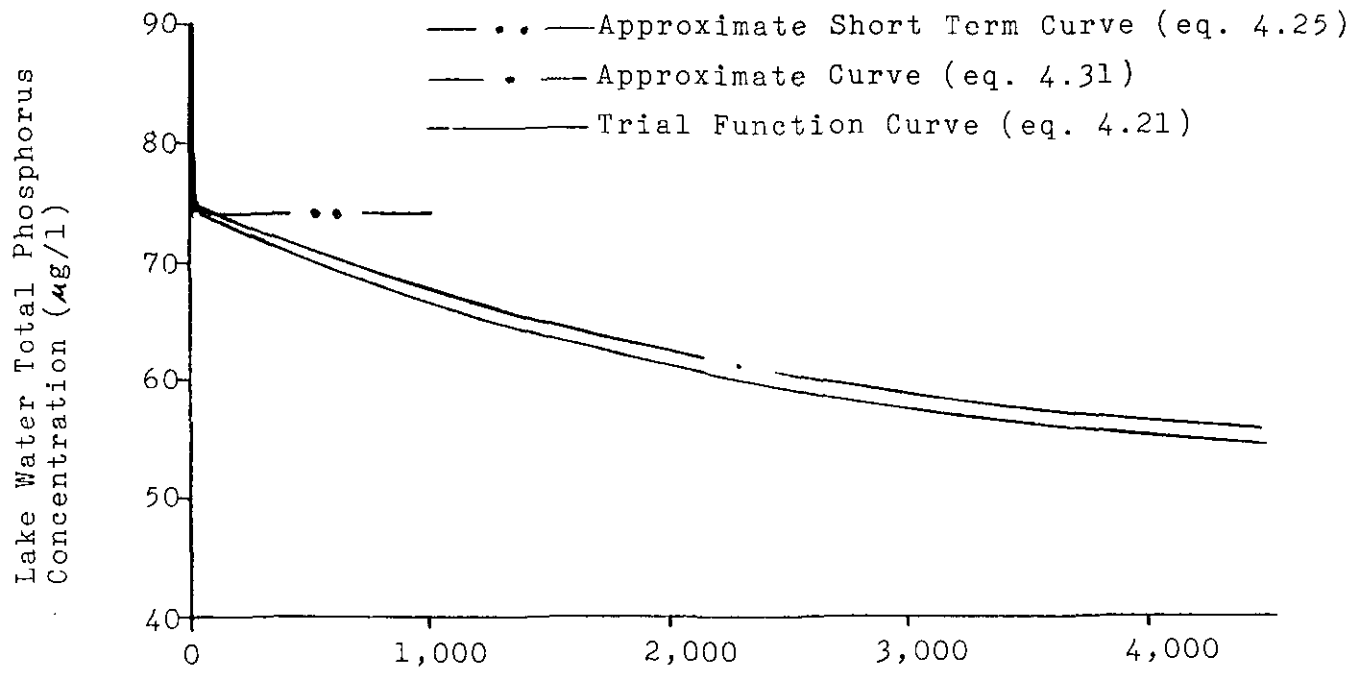


Figure 15. Comparison of model predictions ( $P_I(t)$ ), Trial Function Solution (eq. 4.21), Approximate Short-Term Curve (eq. 4.25) and Approximate Curve (eq. 4.31).

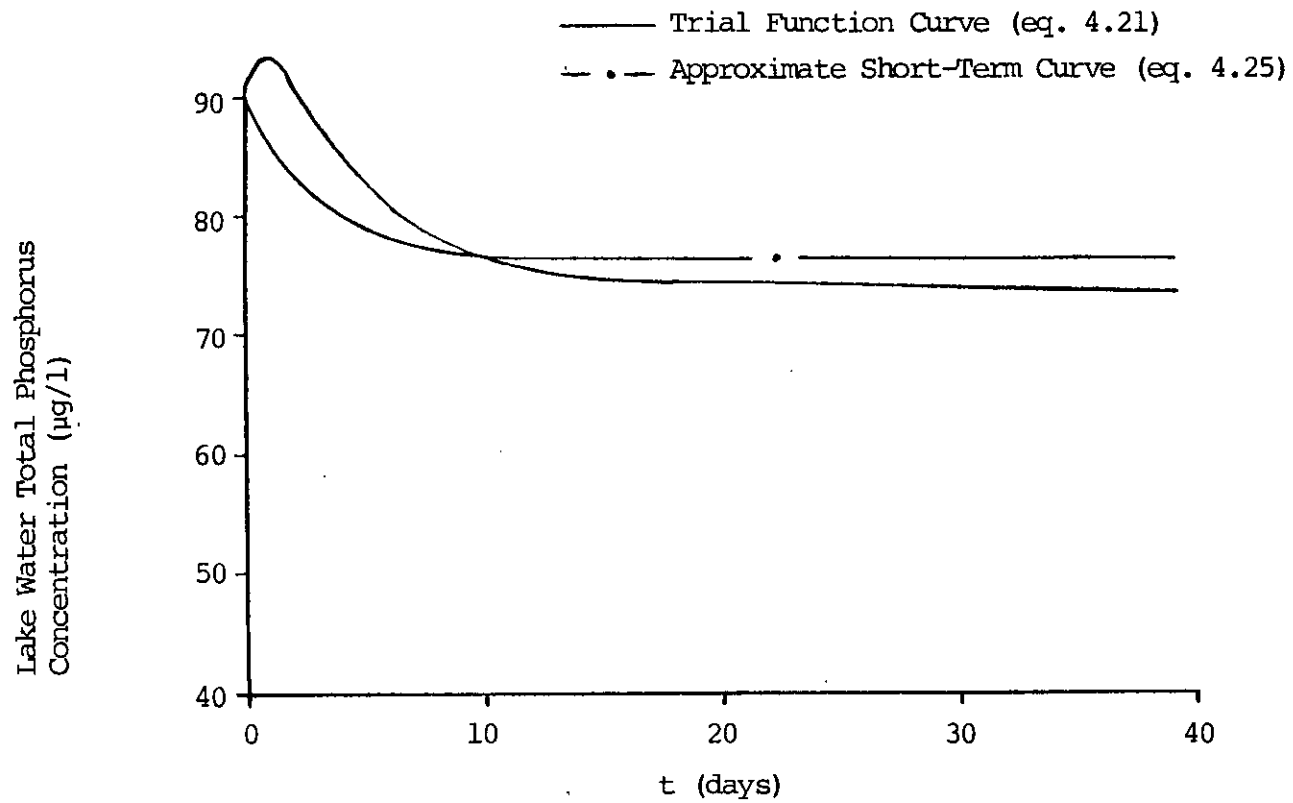


Figure 16. Expanded time scale. Comparison of model predictions ( $P_e(t)$ ), Trial Function Solution (eq. 4.21) and Approximate Solution (eq. 4.25).

Intermediate term predictions. Figure 17, obtained from equation (4.21), shows that after the initial recovery period the lake water phosphorus concentration reaches a quasi-steady state condition. The rate of recovery of the lake is controlled by phosphorus release from the lake bottom, hence, sediment interstitial and solid phase phosphorus concentrations are allowed to change over long time horizons (several years to a decade). In a similar manner in which intermediate chemical species are considered constant to simplify differential rate equations the rate of change of the lake water phosphorus concentration can be considered zero. An algebraic expression for  $P_L$  can be found by setting the left hand side of equation (4.20) equal to zero and solving for  $P_L$ .

$$P_L = -X_8/X_7 - X_6 P_i/X_7 \quad 4.26$$

Substituting equation (4.26) into equations (4.18) and (4.19) yields:

$$\frac{dP_s}{dt} = X_1 P_s - X_2 X_8/X_7 - X_2 X_6 P_i/X_7 \quad 4.27$$

$$\frac{dP_i}{dt} = X_3 P_s + P_i (X_4 - X_5 X_6/X_7) - X_5 X_8/X_7 \quad 4.28$$

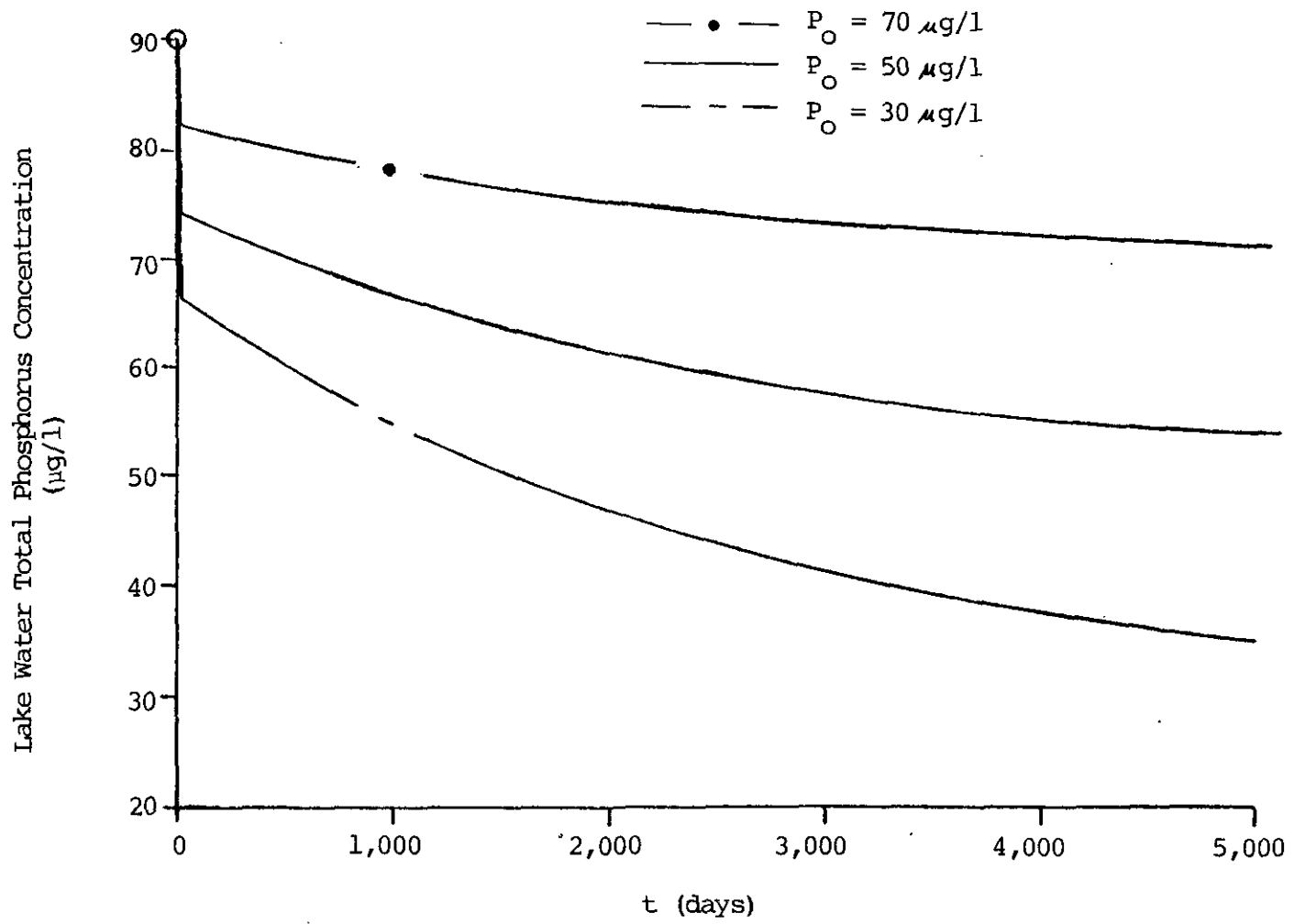


Figure 17. Lake water response to reductions of phosphorus loading ( $P_0$ ) from 90 to 70, 50 and 30 ( $\mu\text{g/l}$ ).

Equations (4.27) and (4.28) can be solved simultaneously by Laplace transform methods to yield:

$$\begin{aligned}
P_i(t) = & \left[ [P_{so} X_3 - P_{io} (G-r)] / 2r \right] e^{-t(H-r)} \\
& - \left[ [P_{so} X_3 - P_i (G+r)] / 2r \right] e^{-t(H+r)} \\
& + \left[ [X_2 X_3 X_8 / X_7 - X_5 X_8 / X_7 (G+r)] / 2r(H+r) \right] [1 - e^{-t(H+r)}] \\
& - \left[ [X_2 X_3 X_8 / X_7 - X_5 X_8 / X_7 (G-r)] / 2r(H-r) \right] [1 - e^{-t(H-r)}]
\end{aligned}$$

4.29

$$\begin{aligned}
P_s(t) = & \left[ [P_{so} (G+r) - P_{io} X_2 X_6 / X_7] / 2r \right] e^{-t(H-r)} \\
& - \left[ [P_{so} (G-r) - P_{io} X_2 X_6 / X_7] / 2r \right] e^{-t(H+r)} \\
& - \left[ X_2 X_8 (X_6 X_5 / X_7^2 - 1) (G-r) / 2r(H+r) X_7 \right] [1 - e^{-t(H+r)}] \\
& + \left[ X_2 X_8 (X_6 X_5 / X_7^2 - 1) (G+r) / 2r(H-r) X_7 \right] [1 - e^{-t(H-r)}]
\end{aligned}$$

4.30

Substituting the equation for  $P_i(t)$  (equation (4.29)) into equation (4.26) yields a time dependent equation for  $P_L$ :

$$\begin{aligned}
P_L(t) = & -X_8 / X_7 - [X_6 / X_7] \left\{ \left[ [P_{so} X_3 - P_{io} (G-r)] / 2r \right] e^{-t(H-r)} \right. \\
& - \left[ [P_{so} X_3 - P_i (G+r)] / 2r \right] e^{-t(H+r)} \\
& + \left[ [X_2 X_3 X_8 / X_7 - X_5 X_8 / X_7 (G+r)] / 2r(H+r) \right] [1 - e^{-t(H+r)}] \\
& \left. - \left[ [X_2 X_3 X_8 / X_7 - X_5 X_8 / X_7 (G-r)] / 2r(H-r) \right] [1 - e^{-t(H-r)}] \right\}
\end{aligned}$$

4.31

where:

$$\begin{aligned}
 G &= (x_5 x_6 - x_4 x_7 + x_1 x_7) / 2x_7 \\
 H &= (x_5 x_6 - x_4 x_7 - x_1 x_7) / 2x_7 \\
 r &= \left[ \left[ (x_5 x_6 - x_4 x_7 - x_1 x_7) / x_7 \right]^2 - 4 \left[ x_2 x_3 x_6 / x_7 - x_1 (x_5 x_6 - x_4 x_7) / x_7 \right] \right]^{1/2}
 \end{aligned}$$

and

$$\begin{aligned}
 x_1 &= -K_3 \\
 x_2 &= K_2 V_L / V_S \\
 x_3 &= K_3 / \epsilon \\
 x_4 &= -AK_1 / V_S \\
 x_5 &= AK_1 / V_S \\
 x_6 &= \epsilon AK_1 / V_L \\
 x_7 &= -(Q + \epsilon AK_1 + K_2 V_L) / V_L \\
 x_8 &= P_0 / T
 \end{aligned}$$

Figures 15, 18 and 19 compare the approximate lake model equations with the solutions obtained from the trial function method. The approximate curves compare favorably with the curves produced by the analytical equations obtained from the trial function method.

Program coding for the use of equations (4.29, 30 and 31) on a TI-59 programmable calculator appear in Appendix C.

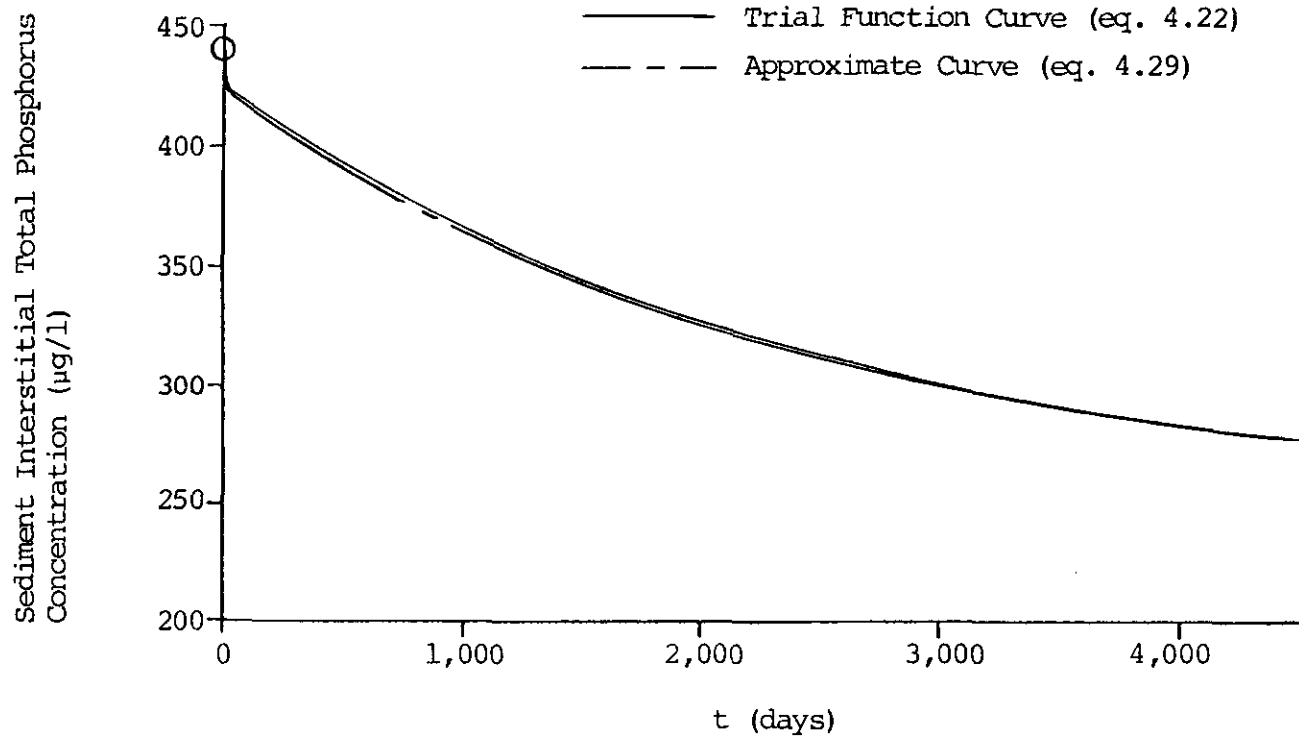


Figure 18. Comparison of model predictions, Trial Function Solution (eq. 4.22) and Approximate Solution (eq. 4.29).



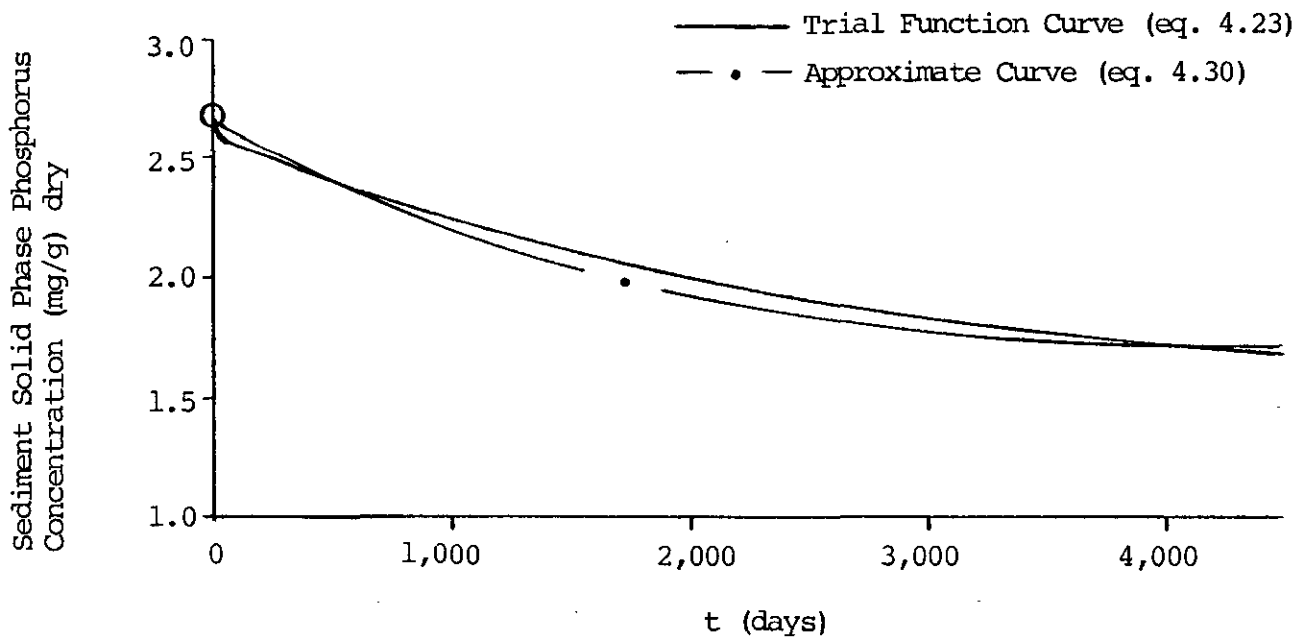


Figure 19. Comparison of model predictions ( $P_s(t)$ ), Trial Function Solution (eq. 4.23) and Approximate Solution<sup>S</sup> (eq. 4.30).

### Sensitivity Analysis

Figures 20 through 24 show sensitivity of the lake model equations (4.21, 22 and 23) to parameters  $K_1$ ,  $K_2$ ,  $K_3$ ,  $Q$  and  $Dr$ . Table 3 shows the initial conditions and parameter values used in the sensitivity analysis. Figure 20 shows that variations of the release rate  $K_1$  by two orders of magnitude has little effect on the prediction of lake water phosphorus concentration. An explanation might be that the sediment desorption rate  $K_3$  controls the release of phosphorus from the lake bottom to the lake water. Sediment solid phase and interstitial phosphorus concentrations are sensitive to variations as small as 1% in the value of  $K_1$ .

Figure 21 shows that a 5% variation in the sedimentation rate ( $K_2$ ) affects the lake water phosphorus concentration in the early stages of the lake recovery. The effect of variations of  $K_2$  diminishes as the time horizon increases. In contrast, varying  $K_2$  affects long term predictions of the sediment solid phase and interstitial phosphorus concentrations.

Figures 22 and 23 show that the sensitivity of the lake water equation to the phosphorus desorption rate ( $K_3$ ) and sediment reactive depth ( $Dr$ ) is comparable to the sensitivity to  $K_2$ . The value of  $K_2$ ,  $K_3$ , or  $Dr$  used is most important when making predictions of less than five years. The sediment interstitial phosphorus concentration is significantly effected by varying the values of  $K_3$  and  $Dr$  ( $t < 5$  years) but grows sensitive to  $K_2$  for

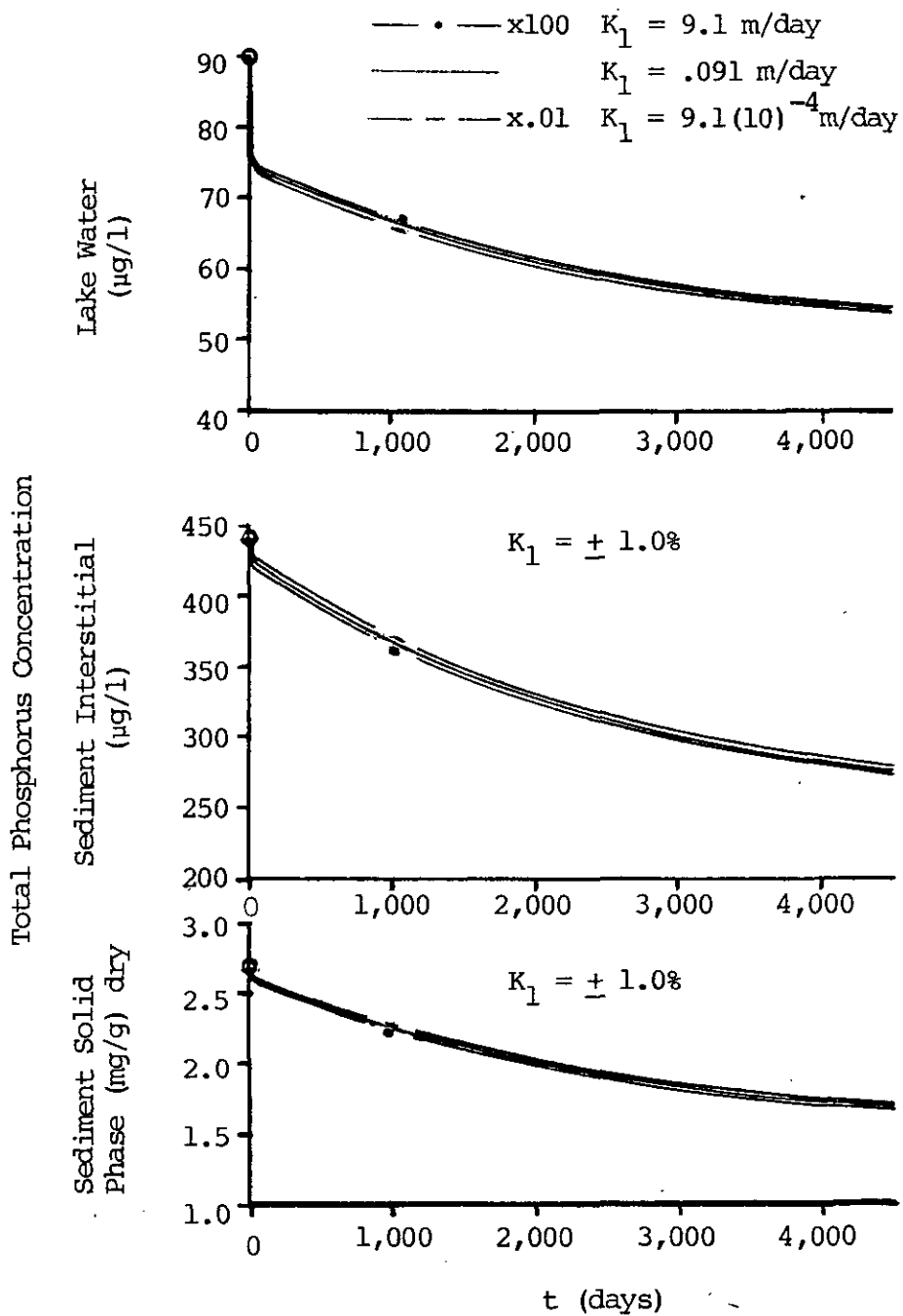


Figure 20. Sensitivity of model to release rate constant  $K_1$ .

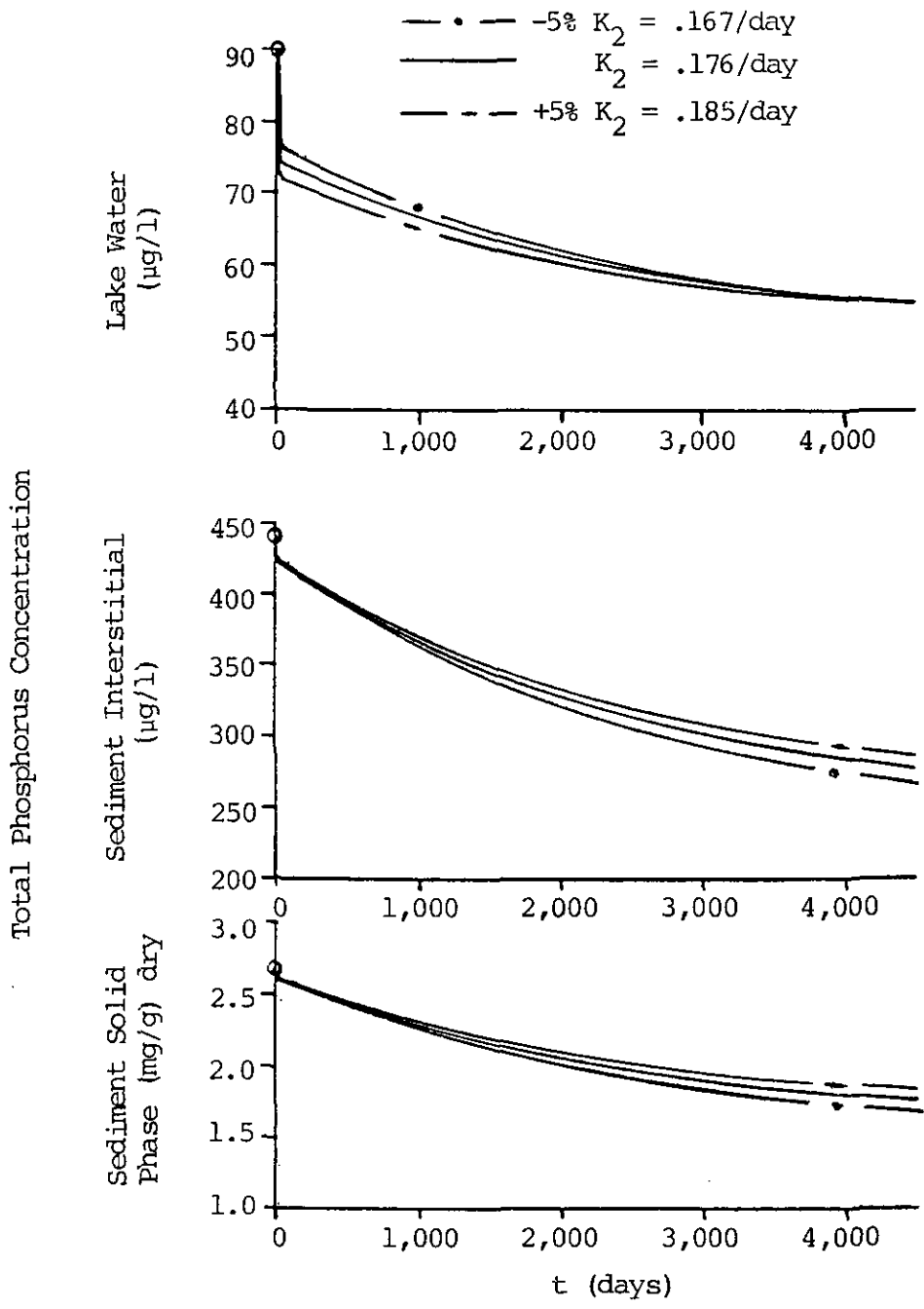


Figure 21. Sensitivity of model to release rate constant  $K_2$ .

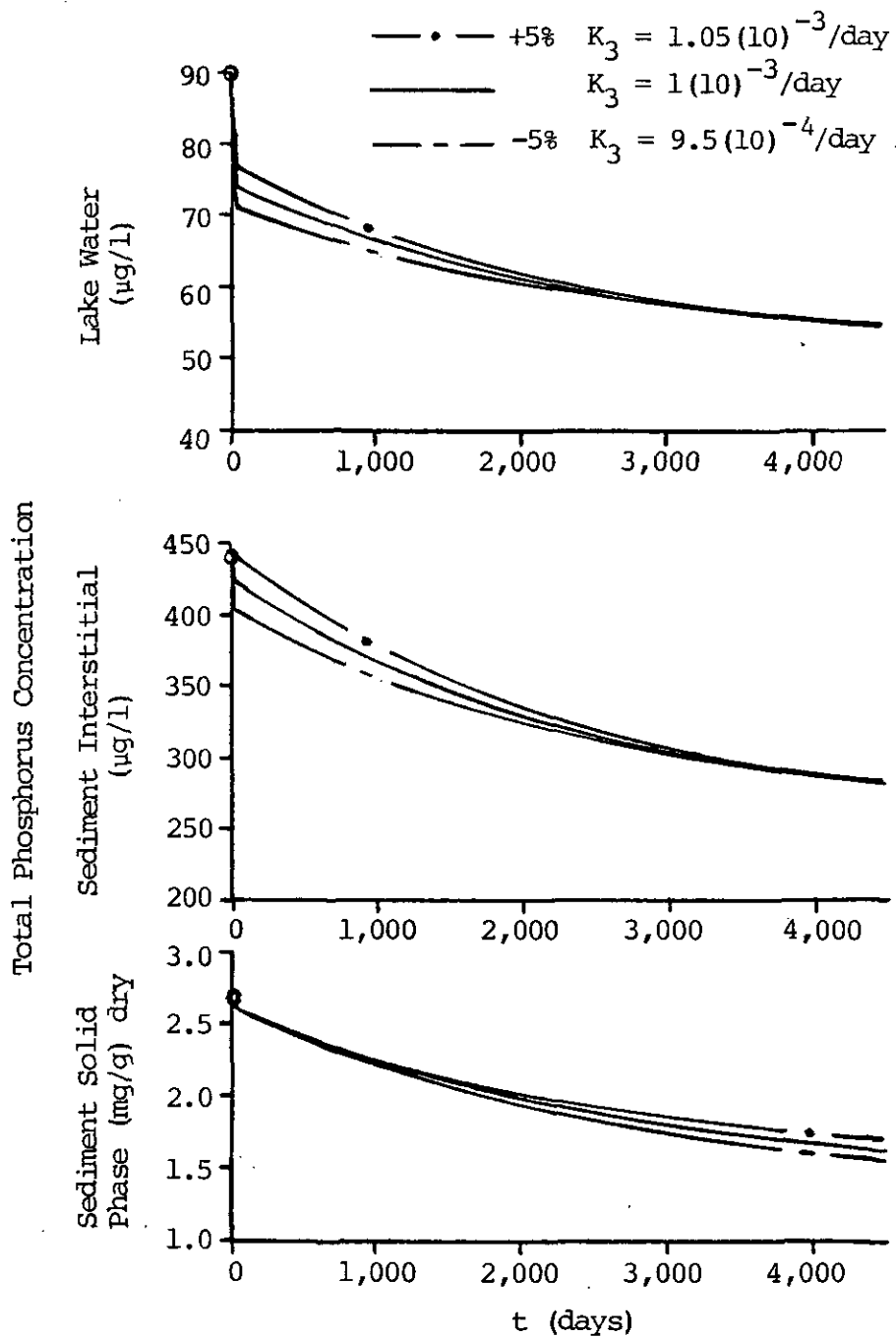


Figure 22. Sensitivity of model predictions to sediment phosphorus desorption rate constant  $K_3$ .

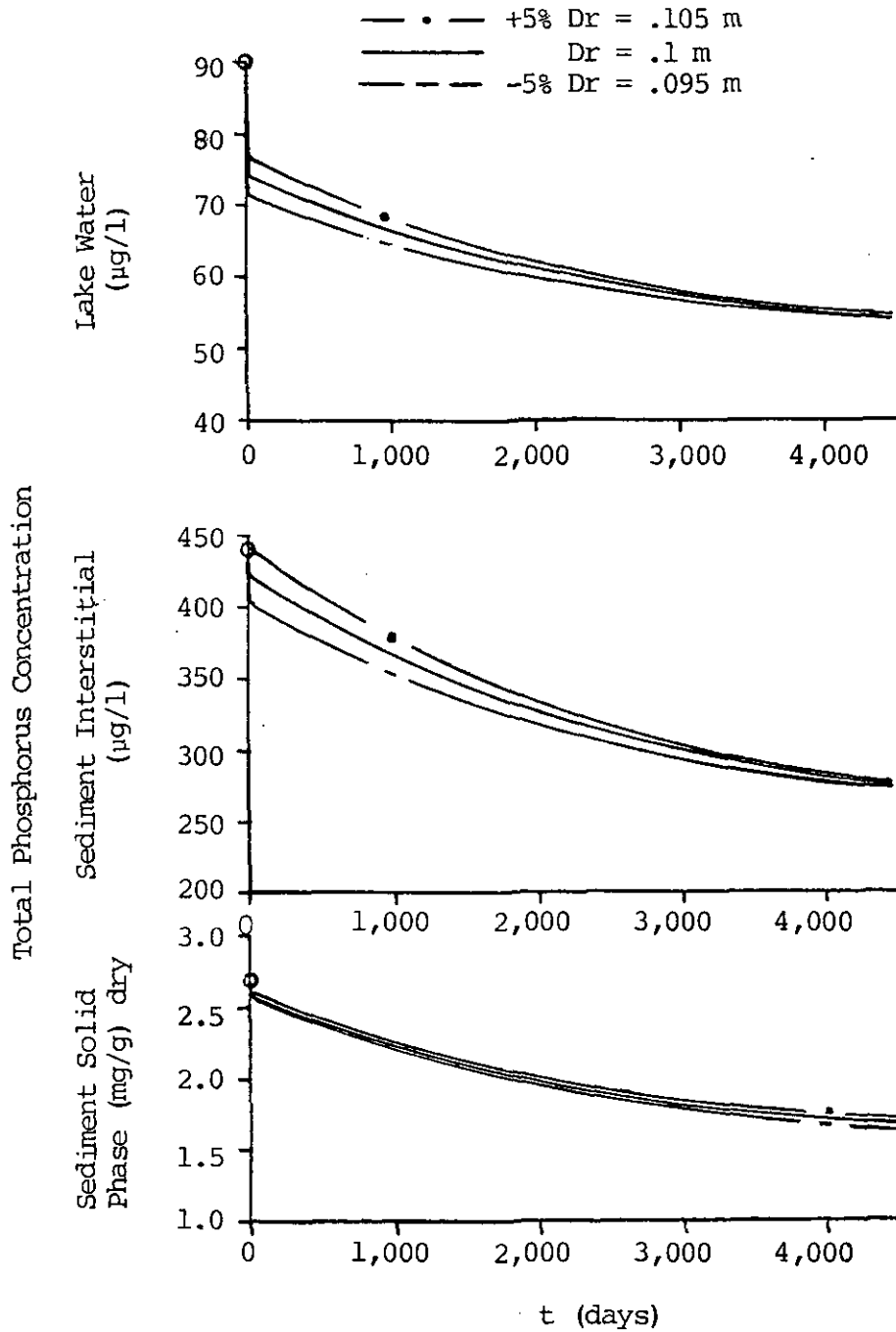


Figure 23. Sensitivity of model to sediment reactive depth  $Dr$ .

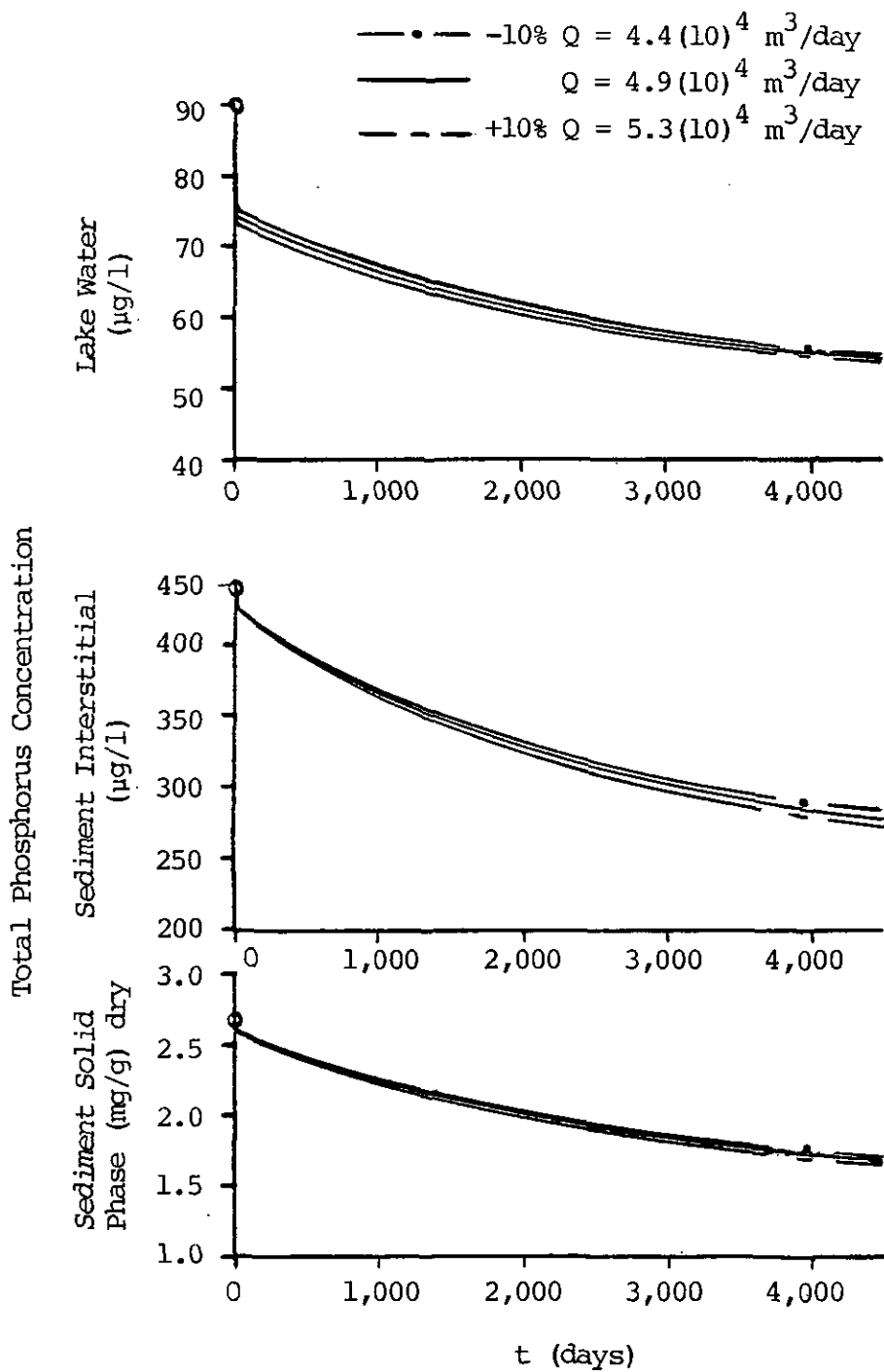


Figure 24. Sensitivity of model to flow rate  $Q$ .

Table 3. Values for initial conditions and model parameters used in sensitivity analysis.

ITEM	VALUE
Initial conditions:	
Lake Water Phosphorus Concentration ( $P_{Lo}$ )	90 $\mu\text{g}/\text{l}$
Sediment Interstitial Water Phosphorus Concentration ( $P_{io}$ )	440.5 $\mu\text{g}/\text{l}$
Sediment Solid Phase Phosphorus Concentration ( $P_{so}$ ) <sup>*</sup>	2.6769 (mg/g) dry
Phosphorus Loading ( $t > 0$ ) ( $P_o$ )	50 $\mu\text{g}/\text{l}$
$K_1$	0.091 m/day
$K_2$	0.176 /day
$K_3$	$10^{-3}$ /day
Dr	0.1 m
Q	$4.89(10)^4 \text{ m}^3/\text{day}$
$V_L$	$4.35(10)^5 \text{ m}^3$
A	$2.572(10)^5 \text{ m}^2$
	0.84
Exchangeable Sediment Solid Phase Phosphorus (% Exch.)	25.0%

\*Sediment solid phase phosphorus concentration ( $P_{so}$ ) is calculated by:

$$P_{so} = P_s (\text{mg/g}) \text{dry} \times 10^6 \times (\% \text{ Exch.}) / (\text{sp. Gr.} = 2.5)$$



$t > 5$  years. Long term predictions of more than 5 years of sediment solid phase phosphorus concentrations are sensitive to the value of  $K_2$  and  $K_3$  chosen.

Varying the flow rate ( $Q$ ) (Figure 24) in the lake model equations produces small changes in predicted phosphorus concentrations. The sensitivity of the lake water equations to  $Q$  is constant over time. The sensitivity of the sediment interstitial and sediment solid phase phosphorus concentration to  $Q$  increases slightly over time.

Prediction of the dynamic response of the lake water phosphorus concentration from 90 to 70, 50 and 30 ( $\mu\text{g/l}$ ) are shown in Figure 17. Significant reduction in lake water phosphorus concentration occurs within a few hydraulic detention times due to dilution. The rate of reduction in phosphorus concentration is controlled thereafter by sedimentation and internal phosphorus cycling.

It is expected that the sensitivity to input parameters of the equations obtained by approximation techniques, would be very similar to the sensitivity characteristics shown by the equations obtained by the trial function method.

## CHAPTER V

## CONCLUSIONS AND RECOMMENDATIONS

Phosphate, an essential plant nutrient, is often limiting to plant growth in natural aquatic ecosystems. Total phosphorus is the best index of the biologically available phosphorus. Mass balance models on lake water total phosphorus are useful for predicting the dynamic response of lake phosphorus concentration to pollution control measures. Eutrophic lake bottoms often contain a significant reservoir of phosphorus which may be released to the overlying water and delay the recovery of eutrophic lakes.

Investigations of the hydrogeologic setting of a lake can provide good "first cut" information on the importance of ground water-lake interaction. Seepage meters can be used to measure the magnitude, direction and distribution of ground water interaction with lakes. However, these devices are not considered reliable for sampling seepage water for chemical analysis.

Lake Warner received wastewater overflows from the Amherst Wastewater Treatment Plant for about ten years and began to exhibit eutrophic characteristics. Wastewater overflows were stopped in November, 1975, making Lake Warner a good site for testing mathematical models for the recovery of a eutrophic lake.

Field measurements on the lake phosphorus budget indicated sediment phosphorus release during high water temperature and low

flow conditions. A cursory hydrogeologic study of Lake Warner suggested that ground water would discharge into the lake on one side and recharge the ground water aquifer on the other side. Field measurements using seepage meters verified this hypothesis, but showed that ground water was an insignificant component of Lake Warner's water and phosphorus budget and could therefore be neglected in the phosphorus mass balance model. Seepage water samples were not considered representative of sediment pore water chemical composition.

A mass balance on Lake Warner phosphorus indicated that the lake sediments released phosphorus on the July 1981 sampling date and retained phosphorus during September 1981.

The analytical expression for the lake recovery model is a valuable tool for predicting the response of completely mixed lakes, with short water residence times, to changes in phosphorus loading. The implementation of the model for planning, management and research purposes has been facilitated by the development of an analytical expression of the model as opposed to numerical solutions which require the use of high speed computers.

The approximate solutions to the lake model equations are more easily programmed and convenient to use on the TI-59 programmable calculator than are the analytical equations obtained from the trial function method. The short term lake

phosphorus equation (equation (4.25)) is very easy to use and can be programmed on calculators with only limited programming capabilities.

The close agreement between the curves obtained by the two solution methods suggests the use of the approximate equations for "first cut" predictions of lake recovery.

Future research on the behavior of conservative materials in deposit-water systems as well as planning and management activities is facilitated by the convenience of the analytical expression of the lake recovery model. Sensitivity analysis on equation (4.21) shows that the value of parameters  $K_2$ ,  $K_3$  and  $D_r$  significantly affect model predictions in the early stages of lake recovery. Care should be taken in estimating the value of these parameters for use in the lake model.

It is recommended that researchers seek to refine the values of reaction rates for particular lakes and that field data used in conjunction with sensitivity analysis be carried out for a wide range of lake systems. As a wider range of lake systems are studied, and more data are accumulated on physical, chemical and biological characteristics of lake bottoms, efforts should be made to compile guidelines which can be employed by model users for choosing the values of reaction rates which would apply to lakes under consideration.

Although beyond the scope of this study it is suggested that efforts be made to relax the assumption of a constant conversion rate ( $K_3$ ) between sediment solid phase phosphorus and sediment

interstitial phosphorus. A time-dependent  $K_3$  would, however, increase the mathematical complexity of the model. Some of the following suggestions might be considered:

- i) Use low values of  $K_3$  for the winter season and high values of  $K_3$  for the summer season. Calculate new parameters for each season. Determine the values of the seasonal  $K_3$  values by field investigations.
- ii) Concentrate on determining the value of  $K_3$  which would accurately depict sediment conditions in the summer. Sediment phosphorus release during the summer supports biological activity and is, therefore, of interest.

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APPENDIX A

1. MILL RIVER STAGE DISCHARGE CURVES
2. ORTHOPHOSPHATE CALIBRATION CURVE
3. TOTAL PHOSPHORUS CALIBRATION CURVE

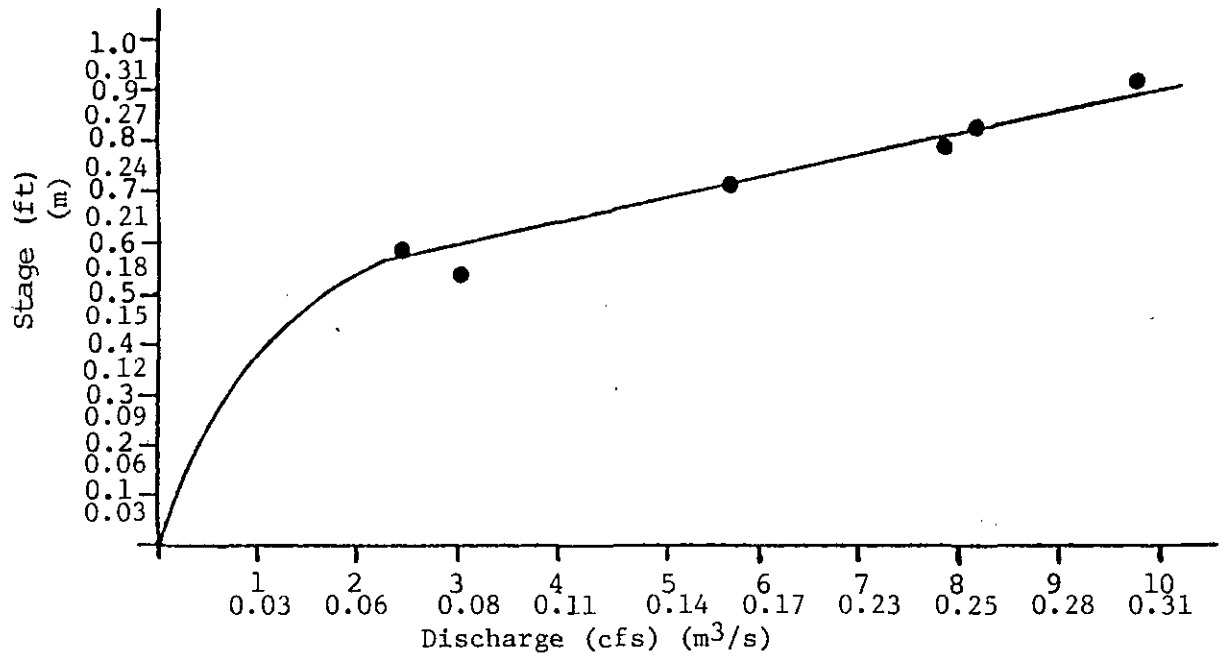


Figure 25. Stage-discharge curve of the Mill River (low flow) at the inlet of Lake Warner.

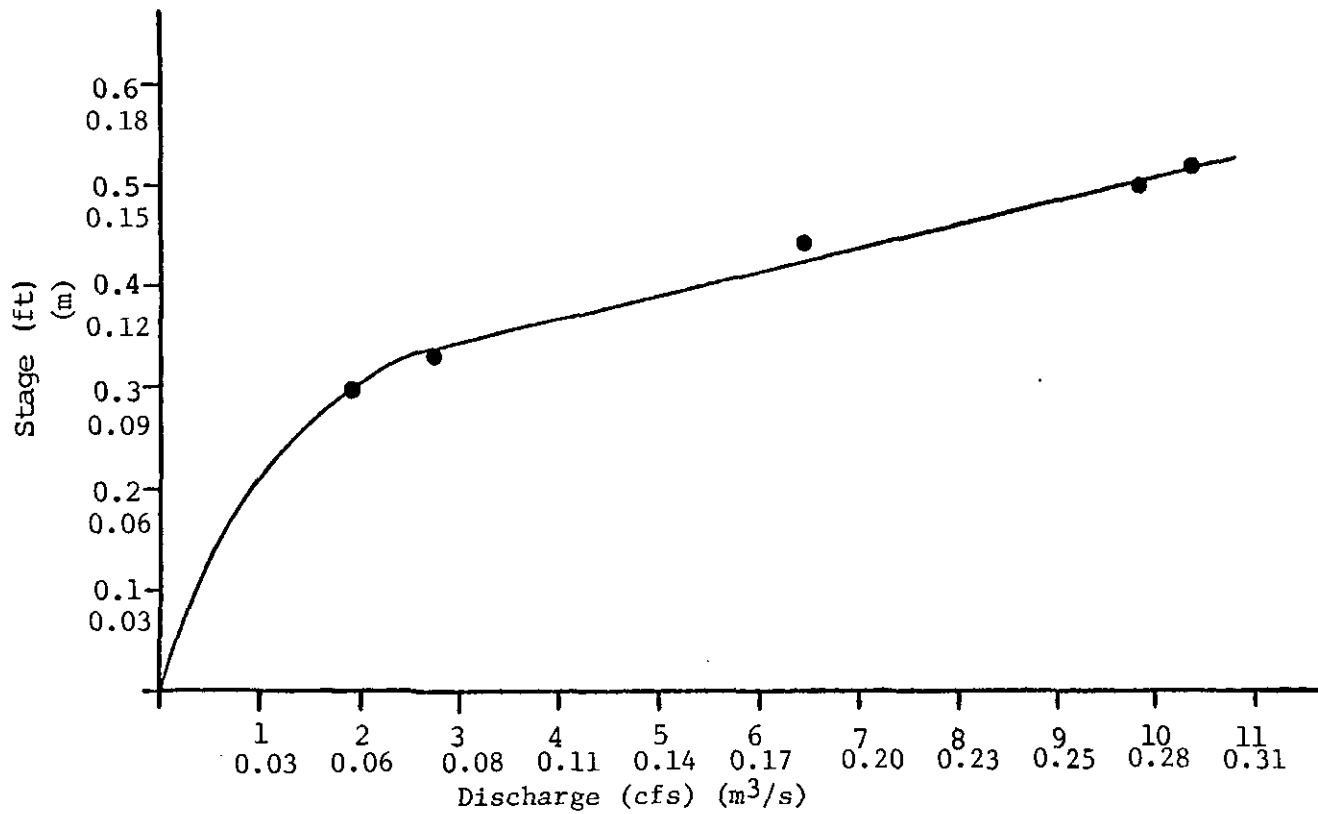


Figure 26. Stage-discharge curve of the Mill River (low flow) at the outlet of Lake Warner.

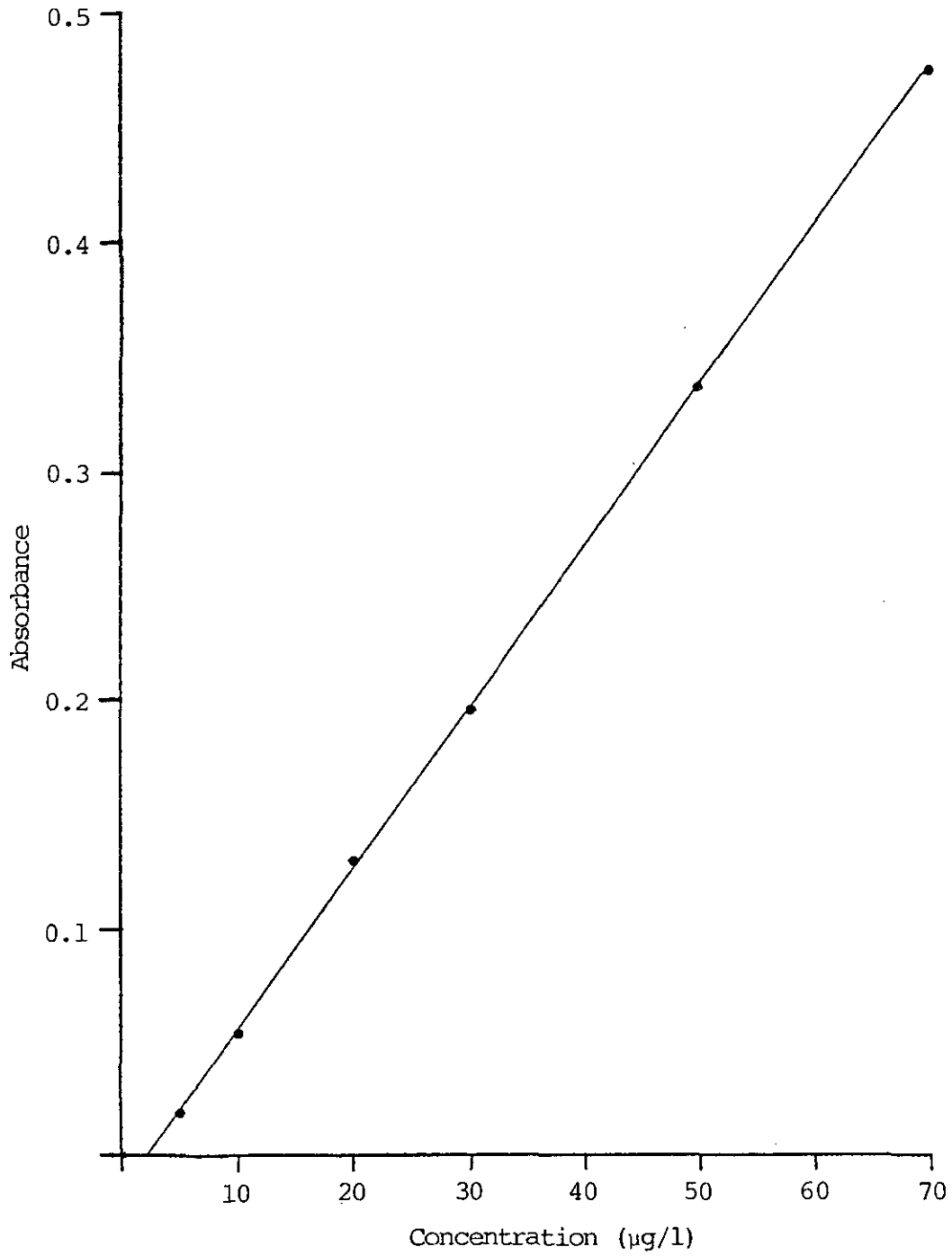


Figure 27. Orthophosphate calibration curve.

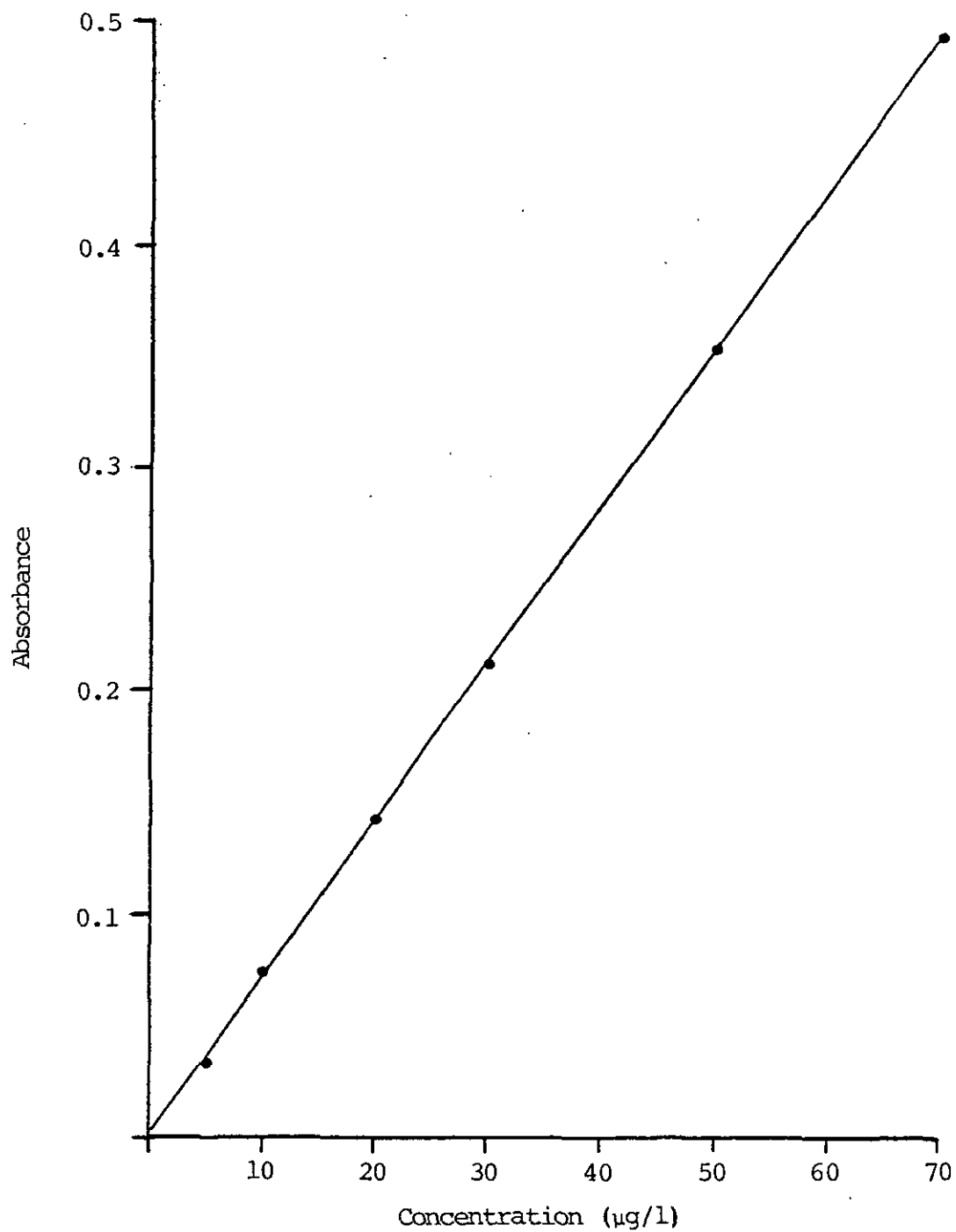


Figure 28. Total phosphorus calibration curve.

APPENDIX B

MATHEMATICAL METHODS



### Introduction

The modified Snow and DiGiano (1976) lake recovery model consists of a system of linear first order ordinary differential equations with constant coefficients. Methods for solving such a system of differential equations include:

- i) The Laplace transform method
- ii) The power series method
- iii) The eigenvalue method
- iv) The trial function method
- v) Numerical methods
- iv) Other methods

Snow and DiGiano (1976) and Bingham and Feng (1980) used numerical methods to solve the lake recovery model. The analytical expression for the lake recovery model presented here was developed using the trial function method. In the trial function method the number of homogeneous differential equations ( $n$ ) to be solved simultaneously is related to the number of algebraic equations ( $N$ ) needed to solve for the unknown coefficients by equation (7.1):

$$N = n^n + n \quad 7.1$$

As can be seen from Table 3 the trial function method was a reasonable approach for the solution of the lake recovery model

as there are three differential equations to be solved simultaneously. These simultaneous differential equations are probably the maximum which can be conveniently solved by the trial function method.

Table 4. Number of algebraic equations needed to solve for the coefficients in a system of homogeneous ordinary differential equations.

Number of Ordinary Differential Equations  n	Number of Simultaneous Algebraic Equations to Solve for the Coefficients  N
1	2
2	6
3	12
4	260
5	3,130
6	46,662

The form of the trial function to be postulated for the solution of homogeneous and nonhomogeneous linear first order ordinary differential equations with constant coefficients is discussed below.

Homogeneous case. Given a simple ordinary differential equation:

$$\frac{dx}{dt} = C_1 x \quad 7.2$$

with initial conditions:

$$X(0) = X_0 \quad 7.3$$

a trial function of the form

$$X(t) = me^{\alpha t} \quad 7.4$$

should be postulated. Taking the derivative of equation (7.4) with respect to  $t$  yields:

$$\frac{dX}{dt} = \alpha me^{\alpha t} \quad 7.5$$

substituting (7.4) and (7.5) into equation (7.2) yields:

$$\alpha me^{\alpha t} = C_1 me^{\alpha t} \quad 7.6$$

Hence, the rate coefficient ( $\alpha$ ) can be found by solving a linear equation:

$$\alpha = C_1 \quad 7.7$$

The initial conditions

$$X(0) = X_0, \quad t = 0 \quad 7.3$$

can be applied to equation (7.4) to yield:

$$X(0) = m \quad 7.8$$

Substituting equation (7.8) into equation (7.3) yields:

$$m = X_0 \quad 7.9$$

The solution for the homogenous equation (7.2), therefore, is:

$$X(t) = X_0 e^{C_1 t} \quad 7.10$$

which is the same equation that one would obtain by integration of equation (7.2) and the application of the initial conditions.

Similarly one would postulate trial functions of the form:

$$X(t) = m_1 e^{\alpha t} + m_2 e^{\beta t} \quad 7.11$$

$$Y(t) = n_1 e^{\alpha t} + n_2 e^{\beta t} \quad 7.12$$

to represent a system of homogeneous ordinary differential equations of the form:

$$\frac{dX}{dt} = C_1 X + C_2 Y \quad 7.13$$

$$\frac{dY}{dt} = C_3 X + C_4 Y \quad 7.14$$

Here, the values of  $\alpha$  and  $\beta$  would be the roots of a quadratic equation.

Nonhomogeneous case. There are two possible forms of trial function that one could postulate to represent the solution to an ordinary nonhomogeneous differential equation of the form

$$\frac{dX}{dt} = C_1 X + C_2 \quad 7.15$$

$$\text{with } X = X_0, \quad t = 0 \quad 7.16$$

$$X = X_\infty, \quad t \rightarrow \infty \quad 7.17$$

One form of trial function that one might postulate as a solution to equation (7.15) is:

$$X(t) = m_1(1 - e^{-\alpha t}) + m_2 e^{\beta t} \quad 7.18$$

where the first term on the right-hand side of equation (7.18) represents the nonhomogeneous portion of the solution to equation (7.15) and the second term allows for a finite initial condition.

Solving the (n) simultaneous algebraic equations to find the coefficients of the trial function that represent (N) ordinary differential equations can be simplified by converting the nonhomogeneous ordinary differential equations to homogeneous ordinary differential equations. This can be done by setting the left-hand side of the ordinary nonhomogeneous equation equal to zero and solving for the dependent variables. A change of variable is then employed such that the nonhomogeneous ordinary differential equation becomes homogeneous. This is the method employed in the solution to the lake recovery model.

The Trial Function Method Applied to  
the Lake Recovery Model

The modified Snow and DiGiano model presented by Bingham and Feng (1980) to which the trial function method is applied is:

Sediment Solid Phase Phosphorus

$$V_s \frac{dP_s}{dt} = V_L K_3 P_L - V_s K_3 P_s \quad 4.13$$

Sediment Interstitial Phosphorus

$$\epsilon V_s \frac{dP_i}{dt} = V_s K_3 P_s - \epsilon A K_1 (P_i - P_L) \quad 4.17$$

Lake Water Phosphorus

$$V_L \frac{dP_L}{dt} = Q P_O - Q P_L + \epsilon A K_1 (P_i - P_L) - V_L K_2 P_L \quad 4.9$$

rearranging

$$\frac{dP_s}{dt} = X_1 P_s + X_2 P_L \quad 4.18$$

$$\frac{dP_i}{dt} = X_3 P_s + X_4 P_i + X_5 P_L \quad 4.19$$

$$\frac{dP_L}{dt} = X_6 P_i + X_7 P_L + X_8 \quad 4.20$$

where

$$x_1 = -K_3$$

$$x_2 = K_2 V_L / V_S$$

$$x_3 = K_3 / \mathcal{E}$$

$$x_4 = -AK_1 / V_S$$

$$x_5 = AK_1 / V_S$$

$$x_6 = \mathcal{E}AK_1 / V_L$$

$$x_7 = -(Q + \mathcal{E}AK_1 + K_2 V_L) / V_L$$

$$x_8 = P_0 Q / V_L$$

These nonhomogeneous ordinary differential equations were converted to homogeneous equations by employing a change of variables so that the function (h) describes the departure from steady state conditions.

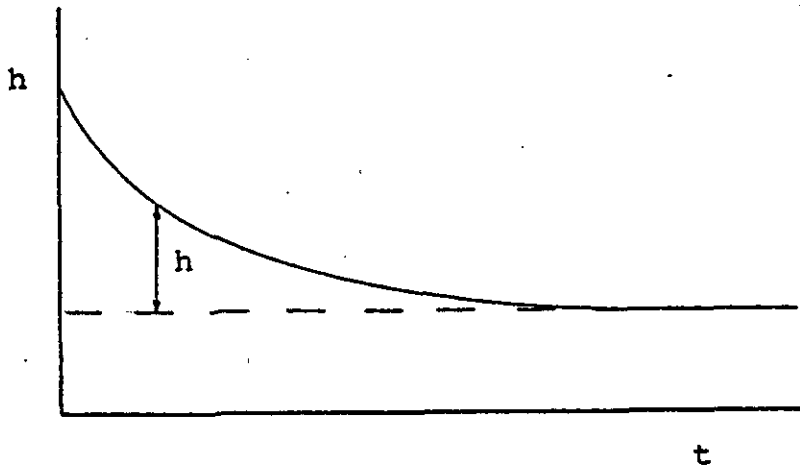


Figure 29. The function h describes the departure from steady state conditions.

$$h_s(t) = P_s(t) - P_{s\infty} \quad 7.19$$

$$h_i(t) = P_i(t) - P_{i\infty} \quad 7.20$$

$$h_L(t) = P_L(t) - P_{L\infty} \quad 7.21$$

Hence, the homogeneous expression for the lake recovery model becomes:

$$\frac{dh_s}{dt} = X_1 h_s + X_2 h_L \quad 7.22$$

$$\frac{dh_i}{dt} = X_3 h_s + X_4 h_i + X_5 h_L \quad 7.23$$

$$\frac{dh_L}{dt} = X_6 h_i + X_7 h_L \quad 7.24$$

The steady state conditions  $P_{s\infty}$ ,  $P_{i\infty}$  and  $P_{L\infty}$  were found by setting the left hand sides of equations (4.13, 4.17 and 4.9) equal to zero and solving for  $P_s$ ,  $P_i$  and  $P_L$ , respectively.

$$P_{s\infty} = K_2 V_L P_0 / K_3 V_s \quad 7.25$$

$$P_{i\infty} = K_2 V_L P_0 / c K_1 A + P_0 \quad 7.26$$

$$P_{L\infty} = P_0 \quad 7.27$$

After a change of variable has been employed (equations 7.19, 20 and 21) to convert nonhomogeneous equations to homogeneous equations the trial function must satisfy the initial conditions:

$$h = h_0, \quad t = 0$$



Hence, the trial function for the sediment solid phase phosphorus concentration ( $h_s(t)$ ), the sediment interstitial phosphorus concentration ( $h_i(t)$ ) and the lake water phosphorus concentration ( $h_L(t)$ ) can be postulated:

$$h_s(t) = S_1 e^{\alpha t} + S_2 e^{\beta t} + S_3 e^{\gamma t} \quad 7.28$$

$$h_i(t) = I_1 e^{\alpha t} + I_2 e^{\beta t} + I_3 e^{\gamma t} \quad 7.29$$

$$h_L(t) = L_1 e^{\alpha t} + L_2 e^{\beta t} + L_3 e^{\gamma t} \quad 7.30$$

with initial conditions

$$h_s(0) = S_1 + S_2 + S_3 \quad 7.31$$

$$h_i(0) = I_1 + I_2 + I_3 \quad 7.32$$

$$h_L(0) = L_1 + L_2 + L_3 \quad 7.33$$

Taking the derivatives of equations (7.28, 29 and 30) yields:

$$\frac{dh_s}{dt} = \alpha S_1 e^{\alpha t} + \beta S_2 e^{\beta t} + \gamma S_3 e^{\gamma t} \quad 7.34$$

$$\frac{dh_i}{dt} = \alpha I_1 e^{\alpha t} + \beta I_2 e^{\beta t} + \gamma I_3 e^{\gamma t} \quad 7.35$$

$$\frac{dh_L}{dt} = \alpha L_1 e^{\alpha t} + \beta L_2 e^{\beta t} + \gamma L_3 e^{\gamma t} \quad 7.36$$

Applying equations (7.28, 29, 30, 34, 35 and 36) to equations (7.22, 23 and 24) yields:

$$\begin{aligned} \alpha S_1 e^{\alpha t} + \beta S_2 e^{\beta t} + \gamma S_3 e^{\gamma t} = X_1 [S_1 e^{\alpha t} + S_2 e^{\beta t} + \\ S_3 e^{\gamma t}] + X_2 [L_1 e^{\alpha t} + L_2 e^{\beta t} + L_3 e^{\gamma t}] \end{aligned} \quad 7.37$$

$$\begin{aligned}
\alpha I_1 e^{\alpha t} + \phi I_2 e^{\phi t} + \gamma I_3 e^{\gamma t} &= X_3 [S_1 e^{\alpha t} + \\
S_2 e^{\phi t} + S_3 e^{\gamma t}] + X_4 [I_1 e^{\alpha t} + I_2 e^{\phi t} + \\
I_3 e^{\gamma t}] + X_5 [L_1 e^{\alpha t} + L_2 e^{\phi t} + L_3 e^{\gamma t}] & \quad 7.38
\end{aligned}$$

$$\begin{aligned}
\alpha L_1 e^{\alpha t} + \phi L_2 e^{\phi t} + \gamma L_3 e^{\gamma t} &= X_6 [I_1 e^{\alpha t} + \\
I_2 e^{\phi t} + I_3 e^{\gamma t}] + X_7 [L_1 e^{\alpha t} + \\
L_2 e^{\phi t} + L_3 e^{\gamma t}] & \quad 7.39
\end{aligned}$$

Equating the coefficients of  $e^{\alpha t}$ ,  $e^{\phi t}$  and  $e^{\gamma t}$  in equations (7.37, 38 and 39) yields three sets of equations.

$$\alpha S_1 = X_1 S_1 + X_2 L_1 \quad 7.40.1$$

$$\alpha I_1 = X_3 S_1 + X_4 I_1 + X_5 L_1 \quad 7.40.2$$

$$\alpha L_1 = X_6 I_1 + X_7 L_1 \quad 7.40.3$$

$$\phi S_2 = X_1 S_2 + X_2 L_2 \quad 7.41.1$$

$$\phi I_2 = X_3 S_2 + X_4 I_2 + X_5 L_2 \quad 7.41.2$$

$$\phi L_2 = X_6 I_2 + X_7 L_2 \quad 7.41.3$$

$$\gamma S_3 = X_1 S_3 + X_2 L_3 \quad 7.42.1$$

$$\gamma I_3 = X_3 S_3 + X_4 I_3 + X_5 L_3 \quad 7.42.2$$

$$\gamma L_3 = X_6 I_3 + X_7 L_3 \quad 7.42.3$$

The above yields 9 equations and 12 unknowns. Three more equations that make the solution possible are the initial conditions:

$$h_{so} = S_1 + S_2 + S_3 \quad 7.31$$

$$h_{io} = I_1 + I_2 + I_3 \quad 7.32$$

$$h_{Lo} = L_1 + L_2 + L_3 \quad 7.33$$

Solving for  $\alpha$ ,  $\mathcal{Q}$  and  $\mathcal{Y}$ . Each set of equations (7.40.1, 2 and 3), (7.41.1, 2 and 3) and (7.42.1, 2 and 3) can be reduced by substitution, to a cubic equation in  $\alpha$ ,  $\mathcal{Q}$  and  $\mathcal{Y}$ , respectively. As an example, equations (7.40.1, 2 and 3) will be solved to arrive at the cubic equation.

Solving for  $S_1$  in equation (7.40.1) yields:

$$S_1 = X_2 L_1 / (\alpha - X_1) \quad 7.43$$

Solving for  $I_1$  equation (7.40.3) yields:

$$I_1 = L_1 (\alpha - X_7) / X_6 \quad 7.44$$

Substituting equations (7.43) and (7.44) into equation (7.40.2) yields a cubic equation in .

$$\begin{aligned} \alpha^3 - (X_1 + X_7 + X_4)\alpha^2 + (X_1 X_7 + X_1 X_4 + X_4 X_7 - X_5 X_6)\alpha \\ + X_1 X_5 X_6 - X_1 X_4 X_7 - X_2 X_3 X_6 = 0 \end{aligned} \quad 4.24$$

Using a similar procedure on equation sets (7.41.1, 2 and 3) and (7.42.1, 2 and 3) identical equations in  $\mathcal{Q}$  and  $\mathcal{Y}$  can be found.

Hence values for  $\alpha$ ,  $\beta$  and  $\gamma$  to be used in the lake recovery model are determined to be the roots of equation (4.24).

The roots of equation (4.24) are found using a trigonometric formulation described by Tuma (1979). The general form of equation (4.24) can be expressed as:

$$ar^3 + br^2 + cr + d = 0 \quad 7.45.1$$

The roots  $r_1$ ,  $r_2$  and  $r_3$  of equation 7.45.1 are;

$$r_1 = 2 \sqrt{|P|/3} \cos \phi/3 - b/3a \quad 7.45.2$$

$$r_2 = -2 \sqrt{|P|/3} \cos (\phi + \pi)/3 - b/3a \quad 7.45.3$$

$$r_3 = -2 \sqrt{|P|/3} \cos (\phi - \pi)/3 - b/3a \quad 7.45.4$$

with

$$P = [3(c/a) - (b/a)^2]/3 \quad 7.45.5$$

$$\phi = \cos^{-1} [-q/2 \sqrt{|P|^3/27}] \quad 7.45.6$$

where

$$q = [2(b/a)^3 - 9(b/a)(c/a) + 27(d/a)]/27$$

Solving for  $P_L(t)$ ,  $P_i(t)$  and  $P_s(t)$ . Finding the solution to equations  $P_L(t)$ ,  $P_i(t)$  and  $P_s(t)$  requires the evaluation of the constants  $L_1$ ,  $L_2$ ,  $L_3$ ,  $I_1$ ,  $I_2$ ,  $I_3$ , and  $S_1$ ,  $S_2$ ,  $S_3$  in equations (7.30), (7.29) and (7.28), respectively. This was done by applying equations (7.40.1) through (7.42.3) to the equations defining the initial conditions (equations 7.31, 32 and 33). For

example, to find the constants  $L_1$ ,  $L_2$  and  $L_3$ , equations (7.40.1) through (7.42.3) are employed to find expressions for the constants  $S_1$ ,  $S_2$ ,  $S_3$ ,  $I_1$ ,  $I_2$  and  $I_3$  in terms of the constants  $L_1$ ,  $L_2$  and  $L_3$ . Hence, for the example of the lake water equations, one obtains:

$$h_{so} = AL_1 + BL_2 + CL_3 \quad 7.46.1$$

$$h_{io} = DL_1 + EL_2 + FL_3 \quad 7.46.2$$

$$h_{Lo} = L_1 + L_2 + L_3 \quad 7.46.3$$

where A through F are defined by equations (4.21.1) through (4.21.6), respectively.

To find  $P_L(t)$  the algebraic equations (7.46.1), (7.46.2) and (7.46.3) must be solved simultaneously to find  $L_1$ ,  $L_2$  and  $L_3$ . Substituting the expressions for  $L_1$ ,  $L_2$  and  $L_3$  into equation (7.30) yields equation (4.21). A similar procedure is used to find the solutions for  $P_i(t)$  and  $P_s(t)$ .

#### Order of Magnitude Analysis

A common method for simplifying a differential equation is to examine the order of magnitude of the terms in the equation. The order of magnitude of parameters in the lake model equations are:

$P_s$	$= O [10^5] \mu\text{g/l}$	- sediment solid phase phosphorus
$P_i$	$= O [10^3] \mu\text{g/l}$	- sediment interstitial phosphorus
$P_L$	$= O [10^2] \mu\text{g/l}$	- lake water phosphorus
$Dr$	$= O [10^{-1}] \text{ m}$	- sediment reactive depth
$Z$	$= O [1] \text{ m}$	- lake mean depth
$T$	$= O [10] \text{ days}$	- lake detention time
$K_1$	$= O [10^{-1}] \text{ m/day}$	- sediment release rate
$K_2$	$= O [10^{-1}] \text{ /day}$	- sedimentation rate
$K_3$	$= O [10^{-3}] \text{ /day}$	- sediment desorption rate
$\mathcal{E}$	$= O [1]$	- sediment porosity

Order of magnitude of the terms in equation (4.18).

$$O [K_3 P_s] = O [10^2]$$

$$O [ZK_2 P_L / Dr] = O [10^2]$$

Hence:

$$O [ZK_2 P_L / Dr] = O [K_3 P_s]$$

Order of magnitude of the terms in equation (4.19).

$$O [K_3 P_s / \mathcal{E}] = O [10^2]$$

$$O [K_1 P_i / Dr] = O [10^3]$$

$$O [K_1 P_L / Dr] = O [10^2]$$

Hence:

$$K_3 P_s / \mathcal{E} \quad , \quad K_1 P_L / Dr \ll K_1 P_i / Dr$$

Order of magnitude of terms in equation (4.20).

$$\begin{aligned} O [P_L/T] &= O [10] \\ O [ \mathcal{E} K_1 P_i / Z ] &= O [10^2] \\ O [ \mathcal{E} K_1 P_L / Z ] &= O [10] \end{aligned}$$

Hence:

$$P_L/T, \mathcal{E} K_1 P_L / Z \ll \mathcal{E} K_1 P_i / Z$$

Order of magnitude analysis indicates that the order of  $P_i$  is greater than the other terms in equation (4.20). This fact allowed the simplification of the lake water equation for intermediate term predictions.

APPENDIX C

CODING FOR LAKE MODEL EQUATIONS

(TEXAS INSTRUMENTS 59 PROGRAMMABLE CALCULATOR)



## PARAMETERS

This program calculates the parameters ( $X_1, X_2, X_3, X_4, X_5, X_6, X_7, \alpha, \beta, \gamma$ ) and values of  $P_{i0}, P_{s0}, P_{i\infty}$  and  $P_{s\infty}$  and stores them in data registers 16, 17, 19 and 20, respectively, for use in the lake recovery model equations  $P_L(t), P_i(t)$  and  $P_s(t)$ .

## USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter constants	$K_1$	STO21	$K_1$
	$K_2$	STO22	$K_2$
	$-K_3$	STO23	$-K_3$
	$\epsilon$	STO24	$\epsilon$
	$D_r$	STO25	$D_r$
	$Q$	STO26	$Q$
	$V_L$	STO27	$V_L$
	$A$	STO28	$A$
	$P_o$	STO29	$P_o$
2. Run Program	$P_{Lo}$	STO15	$P_{Lo}$
		E	$\alpha$
	3. Continue Program	R/S	$\beta$
	4. Continue Program	R/S	$\gamma$

<u>Parameter</u>	<u>DATA REGISTER</u>	<u>Parameter</u>	<u>DATA REGISTER</u>
input:			
$K_1$ m/day	21	$X_3$ /day	06
$K_2$ /day	22	$X_4$ /day	07
$-K_3$ /day	23	$X_5$ /day	08
$\mathcal{C}$	24	$X_6$ /day	09
$D_r$ meters	25	$X_7$ /day	10
$Q$ m <sup>3</sup> /day	26	$P$	30
$V_L$ m <sup>3</sup>	27	$Q$	31
$A$ m <sup>2</sup>	28	$\phi$	32
$P_o$ $\mu$ g/l	29, 18	$b$	34
$P_{Lo}$ $\mu$ g/l	15	$c$	35
$t$ days	00	$d$	36

calculated by  
parameter program:

$P_{i0}$ $\mu$ g/l	16
$P_{s0}$ $\mu$ g/l	17
$P_{L\infty}$ $\mu$ g/l	18
$P_{i\infty}$ $\mu$ g/l	19
$P_{s\infty}$ $\mu$ g/l	20
$\alpha$ /day	01
$\beta$ /day	02
$\gamma$ /day	03
$X_1$ /day	04
$X_2$ /day	05

000	76	LBL	060	43	RCL	120	05	05	180	65	x	240	43	RCL
001	11	R	061	28	28	121	55	+	181	43	RCL	241	35	35
002	43	RCL	062	65	x	122	43	RCL	182	07	07	242	65	x
003	29	29	063	43	RCL	123	09	08	183	85	+	243	03	3
004	42	STD	064	21	21	124	55	+	184	43	RCL	244	75	-
005	19	19	065	55	+	125	43	RCL	185	04	04	245	43	RCL
006	53	(	066	43	RCL	126	24	24	186	65	x	246	34	34
007	43	RCL	067	27	27	127	85	+	187	43	RCL	247	33	X <sup>2</sup>
008	22	22	068	54	)	128	01	1	188	10	10	248	54	)
009	65	x	069	95	=	129	54	)	189	85	+	249	55	+
010	43	RCL	070	42	STD	130	65	x	190	43	RCL	250	03	3
011	27	27	071	09	09	131	43	RCL	191	07	07	251	95	=
012	55	+	072	53	(	132	29	29	192	65	x	252	42	STD
013	43	RCL	073	53	(	133	95	=	193	43	RCL	253	30	30
014	28	28	074	43	RCL	134	42	STD	194	10	10	254	53	(
015	55	+	075	26	26	135	19	19	195	75	-	255	53	(
016	43	RCL	076	85	+	136	43	RCL	196	43	RCL	256	43	RCL
017	25	25	077	43	RCL	137	05	05	197	08	08	257	34	34
018	54	)	078	24	24	138	55	+	198	65	x	258	45	YX
019	95	=	079	65	x	139	43	RCL	199	43	RCL	259	03	3
020	42	STD	080	43	RCL	140	23	23	200	09	09	260	54	)
021	05	05	081	28	28	141	65	x	201	54	)	261	65	x
022	43	RCL	082	65	x	142	43	RCL	202	95	=	262	02	2
023	23	23	083	43	RCL	143	15	15	203	42	STD	263	75	-
024	42	STD	084	21	21	144	94	+/-	204	35	35	264	43	RCL
025	04	04	085	85	+	145	95	=	205	53	(	265	34	34
026	53	(	086	43	RCL	146	42	STD	206	43	RCL	266	65	x
027	43	RCL	087	22	22	147	17	17	207	04	04	267	43	RCL
028	23	23	088	65	x	148	43	RCL	208	65	x	268	35	35
029	55	+	089	43	RCL	149	05	05	209	43	RCL	269	65	x
030	43	RCL	090	27	27	150	55	+	210	08	08	270	09	9
031	24	24	091	54	)	151	43	RCL	211	65	x	271	85	+
032	94	+/-	092	55	+	152	23	23	212	43	RCL	272	43	RCL
033	54	)	093	43	RCL	153	94	+/-	213	09	09	273	36	36
034	95	=	094	27	27	154	65	x	214	75	-	274	65	x
035	42	STD	095	54	)	155	43	RCL	215	43	RCL	275	02	2
036	06	06	096	95	=	156	18	18	216	04	04	276	07	7
037	53	(	097	94	+/-	157	95	=	217	65	x	277	54	)
038	43	RCL	098	42	STD	158	42	STD	218	43	RCL	278	55	+
039	28	28	099	10	10	159	20	20	219	07	07	279	02	2
040	65	x	100	53	(	160	92	RTN	220	65	x	280	07	7
041	43	RCL	101	43	RCL	161	76	LBL	221	43	RCL	281	95	=
042	21	21	102	05	05	162	12	B	222	10	10	282	42	STD
043	55	+	103	55	+	163	53	(	223	75	-	283	31	31
044	43	RCL	104	43	RCL	164	43	RCL	224	43	RCL	284	92	RTN
045	28	28	105	08	08	165	04	04	225	05	05	285	76	LBL
046	55	+	106	55	+	166	85	+	226	65	x	286	14	D
047	43	RCL	107	43	RCL	167	43	RCL	227	43	RCL	287	53	(
048	25	25	108	24	24	168	07	07	228	06	06	288	53	(
049	54	)	109	85	+	169	85	+	229	65	x	289	53	(
050	95	=	110	01	1	170	43	RCL	230	43	RCL	290	43	RCL
051	42	STD	111	54	)	171	10	10	231	09	09	291	30	30
052	08	08	112	65	x	172	54	)	232	54	)	292	94	+/-
053	94	+/-	113	43	RCL	173	94	+/-	233	95	=	293	45	YX
054	42	STD	114	15	15	174	95	=	234	42	STD	294	03	3
055	07	07	115	95	=	175	42	STD	235	36	36	295	54	)
056	53	(	116	42	STD	176	34	34	236	92	RTN	296	53	+
057	43	RCL	117	16	16	177	53	(	237	76	LBL	297	02	2
058	24	24	118	53	(	178	43	RCL	238	13	C	298	07	7
059	65	x	119	43	RCL	179	04	04	239	53	(	299	54	)

Figure 30. Coding for TI-59 Programmable Calculator, Parameters Program.

300	34	FX	360	53	(	0.	00
301	35	1/X	361	43	RCL	-.0003886519	01
302	65	x	362	32	32	-.2702561925	02
303	43	RCL	363	85	+	-.9739707142	03
304	31	31	364	89	*	-0.001	04
305	94	+/-	365	54	)	2.976671851	05
306	55	+	366	55	+	.0011904762	06
307	02	2	367	03	3	-0.91	07
308	54	)	368	54	)	0.91	08
309	70	RAD	369	70	RAD	.0451962483	09
310	22	INV	370	39	COS	-.3336155586	10
311	39	COS	371	65	x	0.	11
312	95	=	372	43	RCL	0.	12
313	42	STD	373	33	33	0.	13
314	32	32	374	94	+/-	0.	14
315	53	(	375	75	-	90.	15
316	43	RCL	376	53	(	440.471568	16
317	30	30	377	43	RCL	267900.4666	17
318	94	+/-	378	34	34	50.	18
319	55	+	379	55	+	244.7064267	19
320	03	3	380	03	3	148833.5925	20
321	54	)	381	54	)	0.091	21
322	34	FX	382	95	=	0.176	22
323	65	x	383	42	STD	-0.001	23
324	02	2	384	02	02	0.84	24
325	95	=	385	91	R/S	0.1	25
326	42	STD	386	53	(	48902.4	26
327	33	33	387	53	(	435000.	27
328	92	RTH	388	43	RCL	257200.	28
329	76	LBL	389	32	32	50.	29
330	15	E	390	75	-	-.2526507749	30
331	11	A	391	89	*	.0335129117	31
332	12	B	392	54	)	2.326247743	32
333	13	C	393	55	+	.5804030496	33
334	14	D	394	03	3	1.244615559	34
335	53	(	395	54	)	0.263705188	35
336	53	(	396	70	RAD	.0001023016	36
337	43	RCL	397	39	COS	0.	37
338	32	32	398	65	x	0.	38
339	55	+	399	43	RCL	0.	39
340	03	3	400	33	33		
341	54	)	401	94	+/-		
342	70	RAD	402	75	-		
343	39	COS	403	53	(		
344	65	x	404	43	RCL		
345	43	RCL	405	34	34		
346	33	33	406	55	+		
347	54	)	407	03	3		
348	75	-	408	54	)		
349	53	(	409	95	=		
350	43	RCL	410	42	STD		
351	34	34	411	03	03		
352	55	+	412	91	R/S		
353	03	3					
354	54	)					
355	95	=					
356	42	STD					
357	01	01					
358	91	R/S					
359	53	(					

Figure 30. (continued)

## LAKE WATER PHOSPHORUS CONCENTRATION

The following program calculates the lake water phosphorus concentration ( $P_L(t)$ ) (equation (4.21)) in response to a change in phosphorus inflow concentration ( $P_O = P_{Lo}$ ,  $t < 0$ ;  $P_O = P_{L\infty}$ ,  $t \geq 0$ ). The parameters  $X_1, X_2, X_3, X_4, X_5, X_6, X_6, X_7, \alpha, \theta$  and  $\gamma$  from the parameters program must be used.

## USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter time (days)	t	ST000	t
2. Run program		E	$P_L(t)$
3. Enter time (days)	t	ST000	t
4. Run program		R/S	$P_L(t)$

000	76	LBL	060	09	09	120	08	09	190	04	04	240	04	04
001	16	R	061	54	)	121	08	09	191	02	02	241	04	04
002	53	(	062	55	+	122	08	09	192	75	-	242	04	04
003	53	(	063	43	RCL	123	04	04	193	43	RCL	243	43	STB
004	53	(	064	06	06	124	04	04	194	04	04	244	04	04
005	43	RCL	065	55	+	125	04	04	195	43	RCL	245	92	RTN
006	01	01	066	43	RCL	126	04	04	196	54	)	246	75	LBL
007	75	-	067	09	09	127	43	RCL	197	54	)	247	33	SIN
008	43	RCL	068	54	)	128	01	01	198	53	+	248	53	(
009	07	07	069	42	STB	129	75	-	199	53	(	249	53	(
010	54	)	070	22	22	130	43	RCL	190	43	RCL	250	43	RCL
011	65	x	071	92	RTN	131	04	04	191	02	02	251	25	25
012	53	(	072	76	LBL	132	54	)	192	75	-	252	75	-
013	43	RCL	073	18	C'	133	54	)	193	43	RCL	253	43	RCL
014	01	01	074	53	(	134	55	+	194	04	04	254	24	24
015	75	-	075	53	(	135	53	(	195	54	)	255	54	)
016	43	RCL	076	53	(	136	43	RCL	196	54	)	256	65	x
017	10	10	077	43	RCL	137	01	01	197	42	STB	257	53	(
018	54	)	078	03	03	138	75	-	198	25	25	258	43	RCL
019	75	-	079	75	-	139	43	RCL	199	92	RTN	259	22	22
020	43	RCL	080	43	RCL	140	07	07	200	76	LBL	260	65	x
021	08	08	081	07	07	141	54	)	201	11	R	261	43	RCL
022	65	x	082	54	)	142	55	+	202	53	(	262	26	26
023	43	RCL	083	65	x	143	53	(	203	53	(	263	75	-
024	09	09	084	53	(	144	43	RCL	204	43	RCL	264	43	RCL
025	54	)	085	43	RCL	145	01	01	205	03	03	265	23	23
026	55	+	086	03	03	146	75	-	206	75	-	266	65	x
027	43	RCL	087	75	-	147	43	RCL	207	43	RCL	267	43	RCL
028	06	06	088	43	RCL	148	04	04	208	04	04	268	25	25
029	55	+	089	10	10	149	54	)	209	54	)	269	54	)
030	43	RCL	090	54	)	150	54	)	210	55	+	270	75	-
031	09	09	091	75	-	151	42	STB	211	43	RCL	271	53	(
032	54	)	092	43	RCL	152	24	24	212	09	09	272	43	RCL
033	42	STB	093	08	08	153	92	RTN	213	54	)	273	25	25
034	21	21	094	65	x	154	76	LBL	214	42	STB	274	75	-
035	92	RTN	095	43	RCL	155	10	E'	215	26	26	275	43	RCL
036	76	LBL	096	09	09	156	53	(	216	92	RTN	276	26	26
037	17	B'	097	54	)	157	53	(	217	65	x	277	54	)
038	53	(	098	55	+	158	43	RCL	218	53	(	278	65	x
039	53	(	099	43	RCL	159	02	02	219	43	RCL	279	53	(
040	53	(	100	06	06	160	75	-	220	03	03	280	43	RCL
041	43	RCL	101	55	+	161	43	RCL	221	75	-	281	22	22
042	02	02	102	43	RCL	162	04	04	222	43	RCL	282	65	x
043	75	-	103	09	09	163	54	)	223	04	04	283	43	RCL
044	43	RCL	104	54	)	164	55	+	224	54	)	284	24	24
045	07	07	105	42	STB	165	43	RCL	225	54	)	285	75	-
046	54	)	106	23	23	166	09	09	226	53	+	286	43	RCL
047	65	x	107	92	RTN	167	54	)	227	53	(	287	31	21
048	53	(	108	76	LBL	168	42	STB	228	43	RCL	288	65	x
049	43	RCL	109	19	D'	169	25	25	229	03	03	289	43	RCL
050	02	02	110	53	(	170	92	RTN	230	75	-	290	25	25
051	75	-	111	53	(	171	65	x	231	43	RCL	291	54	)
052	43	RCL	112	43	RCL	172	53	(	232	07	07	292	54	)
053	10	10	113	01	01	173	43	RCL	233	54	)	293	95	*
054	54	)	114	75	-	174	02	02	234	53	+	294	42	STB
055	75	-	115	43	RCL	175	73	-	235	53	(	295	27	27
056	43	RCL	116	04	04	176	43	RCL	236	43	RCL	296	53	(
057	08	08	117	54	)	177	04	04	237	03	03	297	53	(
058	65	x	118	55	+	178	54	)	238	75	-	298	43	RCL
059	43	RCL	119	43	RCL	179	54	)	239	43	RCL	299	25	25

Figure 31. Coding for TI-59 Programmable Calculator, lake model equation  $P_L(t)$  (4.21) solved by Trial Function Method.

300	75	-	360	43	RCL	420	33	22	480	43	RCL	540	54	)
301	43	RCL	361	22	22	421	65	x	481	21	21	541	22	)
302	25	25	362	65	x	422	43	RCL	482	65	x	542	23	LNX
303	54	)	363	43	RCL	423	26	26	483	43	RCL	543	65	x
304	65	x	364	24	24	424	75	-	484	25	25	544	43	RCL
305	53	(	365	54	)	425	43	RCL	485	75	-	545	26	26
306	43	RCL	366	75	-	426	23	23	486	43	RCL	546	65	x
307	23	23	367	53	(	427	65	x	487	24	24	547	53	(
308	65	x	368	43	RCL	428	43	RCL	488	65	x	548	43	RCL
309	43	RCL	369	24	24	429	25	25	489	43	RCL	549	26	26
310	24	24	370	75	-	430	54	)	490	22	22	550	75	-
311	75	-	371	43	RCL	431	55	+	491	54	)	551	43	RCL
312	43	RCL	372	25	25	432	43	RCL	492	55	+	552	24	24
313	21	21	373	54	)	433	27	27	493	43	RCL	553	54	)
314	65	x	374	65	x	434	85	+	494	29	29	554	55	+
315	43	RCL	375	53	(	435	53	(	495	54	)	555	43	RCL
316	26	26	376	43	RCL	436	43	RCL	496	54	)	556	28	28
317	54	)	377	21	21	437	02	02	497	95	=	557	85	+
318	75	-	378	65	x	438	65	x	498	92	RTN	558	53	(
319	53	(	379	43	RCL	439	43	RCL	499	76	LBL	559	53	(
320	43	RCL	380	26	26	440	00	00	500	13	C	560	43	RCL
321	26	26	381	75	-	441	54	)	501	53	(	561	03	03
322	75	-	382	43	RCL	442	22	INV	502	43	RCL	562	65	x
323	43	RCL	383	23	23	443	23	LNX	503	17	17	563	43	RCL
324	24	24	384	65	x	444	65	x	504	75	-	564	00	00
325	54	)	385	43	RCL	445	43	RCL	505	43	RCL	565	54	)
326	65	x	386	24	24	446	26	26	506	20	20	566	22	INV
327	53	(	387	54	)	447	65	x	507	54	)	567	23	LNX
328	43	RCL	388	54	)	448	53	(	508	65	x	568	65	x
329	23	23	389	95	=	449	43	RCL	509	53	(	569	43	RCL
330	65	x	390	42	STD	450	23	23	510	53	(	570	24	24
331	43	RCL	391	29	29	451	65	x	511	43	RCL	571	65	x
332	25	25	392	61	GTD	452	43	RCL	512	01	01	572	53	(
333	75	-	393	39	CDS	453	24	24	513	65	x	573	43	RCL
334	43	RCL	394	76	LBL	454	75	-	514	43	RCL	574	24	24
335	22	22	395	12	B	455	43	RCL	515	00	00	575	75	-
336	65	x	396	53	(	456	21	21	516	54	)	576	43	RCL
337	43	RCL	397	43	RCL	457	65	x	517	22	INV	577	25	25
338	26	26	398	15	15	458	43	RCL	518	23	LNX	578	54	)
339	54	)	399	75	-	459	26	26	519	65	x	579	55	+
340	54	)	400	43	RCL	460	54	)	520	43	RCL	580	43	RCL
341	95	=	401	18	18	461	55	+	521	25	25	581	29	29
342	42	STD	402	54	)	462	43	RCL	522	65	x	582	54	)
343	28	28	403	65	x	463	28	28	523	53	(	583	54	)
344	53	(	404	53	(	464	85	+	524	43	RCL	584	95	=
345	53	(	405	53	(	465	53	(	525	25	25	585	92	RTN
346	43	RCL	406	43	RCL	466	53	(	526	75	-	586	76	LBL
347	24	24	407	01	01	467	43	RCL	527	43	RCL	587	14	D
348	75	-	408	65	x	468	03	03	528	26	26	588	53	(
349	43	RCL	409	43	RCL	469	65	x	529	54	)	589	43	RCL
350	26	26	410	00	00	470	43	RCL	530	55	+	590	16	16
351	54	)	411	54	)	471	00	00	531	43	RCL	591	75	-
352	65	x	412	22	INV	472	54	)	532	27	27	592	43	RCL
353	53	(	413	23	LNX	473	22	INV	533	85	+	593	19	19
354	43	RCL	414	65	x	474	23	LNX	534	53	(	594	54	)
355	21	21	415	43	RCL	475	65	x	535	43	RCL	595	94	+/-
356	65	x	416	25	25	476	43	RCL	536	02	02	596	65	x
357	43	RCL	417	65	x	477	24	24	537	65	x	597	53	(
358	25	25	418	53	(	478	65	x	538	43	RCL	598	53	(
359	75	-	419	43	RCL	479	53	(	539	00	00	599	43	RCL

Figure 31. (continued)

600	01	01	660	53	(	10000.	00
601	65	x	661	43	RCL	-.0003336519	01
602	43	RCL	662	21	21	-.2702561925	02
603	00	00	663	75	-	-.9739707142	03
604	54	)	664	43	RCL	-0.001	04
605	22	INV	665	22	22	2.376671851	05
606	23	LNK	666	54	)	.0011904762	06
607	65	x	667	55	+	-0.91	07
608	43	RCL	668	43	RCL	0.91	08
609	25	25	669	29	29	.0451962483	09
610	65	x	670	54	)	-.3336155586	10
611	53	(	671	54	)	0.	11
612	43	RCL	672	95	=	0.	12
613	22	22	673	92	RTN	0.	13
614	75	-	674	76	LBL	0.	14
615	43	RCL	675	15	E	90.	15
616	23	23	676	16	A*	440.471569	16
617	54	)	677	17	B*	267696.	17
618	55	+	678	18	C*	50.	18
619	43	RCL	679	19	D*	244.7064267	19
620	27	27	680	10	E*	148333.5925	20
621	85	+	681	11	A	.0006113431	21
622	53	(	682	61	GTD	-.2692561925	22
623	43	RCL	683	38	SIN	-.9729707142	23
624	02	02	684	76	LBL	.0012467543	24
625	65	x	685	39	CDS	.0013563243	25
626	43	RCL	686	43	RCL	-.0136027542	26
627	00	00	687	18	18	0.00000558	27
628	54	)	688	85	+	-.0000559628	28
629	22	INV	689	12	B	.0000051292	29
630	23	LNK	690	85	+		
631	65	x	691	13	C		
632	43	RCL	692	85	+		
633	26	26	693	14	D		
634	65	x	694	95	=		
635	53	(	695	91	R/S		
636	43	RCL	696	61	GTD		
637	23	23	697	39	CDS		
638	75	-					
639	43	RCL					
640	21	21					
641	54	)					
642	55	+					
643	43	RCL					
644	28	28					
645	85	+					
646	53	(					
647	53	(					
648	43	RCL					
649	03	03					
650	65	x					
651	43	RCL					
652	00	00					
653	54	)					
654	22	INV					
655	23	LNK					
656	65	x					
657	43	RCL					
658	24	24					
659	65	x					

Figure 31. (continued)



SEDIMENT INTERSTITIAL PHOSPHORUS CONCENTRATION

The following program calculates the sediment interstitial phosphorus concentration ( $P_i(t)$ ) (equation (4.22)) in response to a change in the phosphorus inflow concentration ( $P_o = P_{Lo}, t < 0$ ;  $P_o = P_{L\infty}, t \geq 0$ ). The parameters  $X_1, X_2, X_3, X_4, X_5, X_6, X_7, \alpha, \phi$  and  $\gamma$  from the parameters program must be used.

USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter time (days)	t	STO00	t
2. Run program		E	$P_i(t)$
3. Enter time (days)	t	STO00	t
4. Run program		R/S	$P_i(t)$

000	76	LBL	060	53	(	120	43	RCL	180	09	09	240	55	-
001	16	H'	061	43	RCL	121	09	09	181	54	)	241	43	RCL
002	53	(	062	05	05	122	54	)	182	55	+	242	08	08
003	43	RCL	063	65	x	123	55	+	183	43	RCL	243	55	+
004	05	05	064	43	RCL	124	43	RCL	184	08	08	244	53	(
005	65	x	065	09	09	125	08	08	185	55	+	245	43	RCL
006	43	RCL	066	53	+	126	53	+	186	53	(	246	03	03
007	09	09	067	53	(	127	53	(	187	43	RCL	247	75	-
008	55	+	068	43	RCL	129	43	RCL	188	02	02	248	43	RCL
009	53	(	069	03	03	129	01	01	189	75	-	249	04	04
010	43	RCL	070	75	-	130	75	-	190	43	RCL	250	54	)
011	01	01	071	43	RCL	131	43	RCL	191	04	04	251	55	+
012	75	-	072	04	04	132	04	04	192	54	)	252	53	(
013	43	RCL	073	54	)	133	54	)	193	55	+	253	43	RCL
014	04	04	074	55	+	134	55	+	194	53	(	254	03	03
015	54	)	075	53	(	135	53	(	195	43	RCL	255	75	-
016	55	+	076	43	RCL	136	43	RCL	196	02	02	256	43	RCL
017	53	(	077	03	03	137	01	01	197	75	-	257	10	10
018	43	RCL	078	75	-	138	75	-	198	43	RCL	258	54	)
019	01	01	079	43	RCL	139	43	RCL	199	10	10	259	95	=
020	75	-	080	10	10	140	10	10	200	54	)	260	42	STD
021	43	RCL	081	54	)	141	54	)	201	95	=	261	26	26
022	10	10	082	54	)	142	95	=	202	42	STD	262	92	RTN
023	54	)	083	95	=	143	42	STD	203	25	25	263	76	LBL
024	54	)	084	42	STD	144	24	24	204	76	LBL	264	38	SIN
025	95	=	085	23	23	145	92	RTN	205	11	A	265	53	(
026	42	STD	086	92	RTN	146	76	LBL	206	53	(	266	53	(
027	21	21	087	76	LBL	147	10	E'	207	53	(	267	43	RCL
028	92	RTN	088	19	D'	148	53	(	208	43	RCL	268	25	25
029	76	LBL	089	53	(	149	53	(	209	03	03	269	75	-
030	17	B'	090	53	(	150	43	RCL	210	75	-	270	43	RCL
031	53	(	091	43	RCL	151	02	02	211	43	RCL	271	24	24
032	43	RCL	092	01	01	152	75	-	212	04	04	272	54	)
033	05	05	093	75	-	153	43	RCL	213	54	)	273	65	x
034	65	x	094	43	RCL	154	04	04	214	65	x	274	53	(
035	43	RCL	095	04	04	155	54	)	215	53	(	275	43	RCL
036	09	09	096	54	)	156	65	x	216	43	RCL	276	22	22
037	55	+	097	65	x	157	53	(	217	03	03	277	65	x
038	53	(	098	53	(	158	43	RCL	218	75	-	278	43	RCL
039	43	RCL	099	43	RCL	159	02	02	219	43	RCL	279	26	26
040	02	02	100	01	01	160	75	-	220	07	07	280	75	-
041	75	-	101	75	-	161	43	RCL	221	54	)	281	43	RCL
042	43	RCL	102	43	RCL	162	07	07	222	65	x	282	23	23
043	04	04	103	07	07	163	54	)	223	53	(	283	65	x
044	54	)	104	54	)	164	65	x	224	43	RCL	284	43	RCL
045	55	+	105	65	x	165	53	(	225	03	03	285	25	25
046	53	(	106	53	(	166	43	RCL	226	75	-	286	54	)
047	43	RCL	107	43	RCL	167	02	02	227	43	RCL	287	75	-
048	02	02	108	01	01	168	75	-	228	10	10	288	53	(
049	75	-	109	75	-	169	43	RCL	229	54	)	289	43	RCL
050	43	RCL	110	43	RCL	170	10	10	230	75	-	290	25	25
051	10	10	111	10	10	171	54	)	231	43	RCL	291	75	-
052	54	)	112	54	)	172	75	-	232	05	05	292	43	RCL
053	54	)	113	75	-	173	43	RCL	233	65	x	293	26	26
054	95	=	114	43	RCL	174	05	05	234	43	RCL	294	54	)
055	42	STD	115	05	05	175	65	x	235	06	06	295	65	x
056	22	22	116	65	x	176	43	RCL	236	65	x	296	53	(
057	92	RTN	117	43	RCL	177	06	06	237	43	RCL	297	43	RCL
058	76	LBL	118	06	06	178	65	x	238	09	09	298	22	22
059	18	C'	119	65	x	179	43	RCL	239	54	)	299	65	x

Figure 32. Coding for TI-59 Programmable Calculator, Lake model equation  $P_1(t)$  (4.22) solved by Trial Function Method<sup>1</sup>.

350	43	RCL	360	29	29	430	65	x	480	28	28	540	53	(
351	21	24	361	53	(	421	53	(	481	85	+	541	43	RCL
352	43	-	362	53	(	422	53	(	482	53	(	542	35	25
353	43	RCL	363	43	RCL	423	43	RCL	483	53	(	543	75	-
354	21	21	364	24	24	424	01	01	484	43	RCL	544	43	RCL
355	25	x	365	75	-	425	65	x	485	03	03	545	26	26
356	43	RCL	366	43	RCL	426	43	RCL	486	65	x	546	54	)
357	25	25	367	26	26	427	00	00	487	43	RCL	547	55	+
358	54	)	368	54	)	428	54	)	488	00	00	548	43	RCL
359	54	)	369	65	x	429	22	INV	489	54	)	549	27	27
360	95	=	370	53	(	430	23	LNX	490	22	INV	550	85	+
361	42	STD	371	43	RCL	431	65	x	491	23	LNX	551	53	(
362	27	27	372	21	21	432	43	RCL	492	65	x	552	43	RCL
363	53	(	373	65	x	433	25	25	493	43	RCL	553	02	02
364	53	(	374	43	RCL	434	65	x	494	24	24	554	65	x
365	43	RCL	375	25	25	435	53	(	495	65	x	555	43	RCL
366	26	26	376	75	-	436	43	RCL	496	53	(	556	00	00
367	75	-	377	43	RCL	437	22	22	497	43	RCL	557	54	)
368	43	RCL	378	22	22	438	65	x	498	21	21	558	22	INV
369	25	25	379	65	x	439	43	RCL	499	65	x	559	23	LNX
370	54	)	380	43	RCL	440	26	26	500	43	RCL	560	65	x
371	65	x	381	24	24	441	75	-	501	25	25	561	43	RCL
372	53	(	382	54	)	442	43	RCL	502	75	-	562	26	26
373	43	RCL	383	75	-	443	23	23	503	43	RCL	563	65	x
374	23	23	384	53	(	444	65	x	504	24	24	564	53	(
375	65	x	385	43	RCL	445	43	RCL	505	65	x	565	43	RCL
376	43	RCL	386	24	24	446	25	25	506	43	RCL	566	26	26
377	24	24	387	75	-	447	54	)	507	22	22	567	75	-
378	75	-	388	43	RCL	448	55	+	508	54	)	568	43	RCL
379	43	RCL	389	25	25	449	43	RCL	509	55	+	569	24	24
380	21	21	390	54	)	450	27	27	510	43	RCL	570	54	)
381	65	x	391	65	x	451	85	+	511	29	29	571	55	+
382	43	RCL	392	53	(	452	53	(	512	54	)	572	43	RCL
383	26	26	393	43	RCL	453	43	RCL	513	54	)	573	28	28
384	54	)	394	21	21	454	02	02	514	95	=	574	85	+
385	75	-	395	65	x	455	65	x	515	92	RTN	575	53	(
386	53	(	396	43	RCL	456	43	RCL	516	76	LBL	576	53	(
387	43	RCL	397	26	26	457	00	00	517	13	C	577	43	RCL
388	26	26	398	75	-	458	54	)	518	53	(	578	03	03
389	75	-	399	43	RCL	459	22	INV	519	43	RCL	579	65	x
390	43	RCL	400	23	23	460	23	LNX	520	17	17	580	43	RCL
391	24	24	401	65	x	461	65	x	521	75	-	581	00	00
392	54	)	402	43	RCL	462	43	RCL	522	43	RCL	582	54	)
393	65	x	403	24	24	463	26	26	523	20	20	583	22	INV
394	53	(	404	54	)	464	65	x	524	54	)	584	23	LNX
395	43	RCL	405	54	)	465	53	(	525	65	x	585	55	x
396	23	23	406	95	=	466	43	RCL	526	53	(	586	43	RCL
397	65	x	407	42	STD	467	23	23	527	53	(	587	24	24
398	43	RCL	408	29	29	468	65	x	528	43	RCL	588	65	x
399	25	25	409	61	STD	469	43	RCL	529	01	01	589	53	(
400	75	-	410	39	CDS	470	24	24	530	65	x	590	43	RCL
401	43	RCL	411	76	LBL	471	75	-	531	43	RCL	591	24	24
402	22	22	412	12	B	472	43	RCL	532	00	00	592	75	-
403	55	x	413	53	(	473	21	21	533	54	)	593	43	RCL
404	43	RCL	414	43	RCL	474	65	x	534	22	INV	594	25	25
405	26	26	415	16	16	475	43	RCL	535	23	LNX	595	54	)
406	54	)	416	75	-	476	26	26	536	65	x	596	55	+
407	54	)	417	43	RCL	477	54	)	537	43	RCL	597	43	RCL
408	95	=	418	19	19	478	55	-	538	25	25	598	29	29
409	42	STD	419	64	)	479	43	RCL	539	65	x	599	54	)

Figure 32. (continued)

600	54	)	660	43	RCL	50.	00
601	25	*	661	23	23	- .00153075+2	01
602	92	RTN	662	25	+	- .2107039075	02
603	76	LBL	663	53	(	-1.320771587	03
604	14	D	664	53	(	-0.00186	04
605	53	(	665	43	RCL	2.976671851	05
606	43	RCL	666	03	03	.0022142857	06
607	15	15	667	65	x	-0.91	07
608	75	-	668	43	RCL	0.91	08
609	43	RCL	669	00	00	.0451962483	09
610	18	18	670	54	)	-1.221196248	10
611	54	)	671	22	INV	0.	11
612	94	+/-	672	23	LNx	0.	12
613	65	x	673	65	x	0.	13
614	53	(	674	43	RCL	0.	14
615	53	(	675	24	24	90.	15
616	43	RCL	676	65	x	450.	16
617	01	01	677	53	(	144032.5089	17
618	65	x	678	43	RCL	50.	18
619	43	RCL	679	21	21	213.5533984	19
620	00	00	680	75	-	80018.0605	20
621	54	)	681	43	RCL	395.0243007	21
622	22	INV	682	22	22	- .4051945258	22
623	23	LNx	683	54	)	1.024391278	23
624	65	x	684	55	+	.0370577801	24
625	43	RCL	685	43	RCL	.1101025373	25
626	25	25	686	29	29	- .4539899797	26
627	65	x	687	54	)	24.54349088	27
628	53	(	688	54	)	-101.1738179	28
629	43	RCL	689	95	=	8.260729584	29
630	22	22	690	92	RTN		
631	75	-	691	76	LBL		
632	43	RCL	692	15	E		
633	23	23	693	16	A'		
634	54	)	694	17	B'		
635	55	+	695	18	C'		
636	43	RCL	696	19	D'		
637	27	27	697	10	E'		
638	85	+	698	11	A		
639	53	(	699	61	GTD		
640	43	RCL	700	38	SIN		
641	02	02	701	76	LBL		
642	65	x	702	39	CDS		
643	43	RCL	703	43	RCL		
644	00	00	704	19	19		
645	54	)	705	85	+		
646	22	INV	706	12	B		
647	23	LNx	707	85	+		
648	65	x	708	13	C		
649	43	RCL	709	85	+		
650	26	26	710	14	D		
651	65	x	711	95	=		
652	53	(	712	91	R/S		
653	43	RCL	713	61	GTD		
654	23	23	714	39	CDS		
655	75	-					
656	43	RCL					
657	21	21					
658	54	)					
659	55	+					

Figure 32. (continued)

SEDIMENT SOLID PHASE PHOSPHORUS CONCENTRATION

The following program calculates the sediment solid phase phosphorus concentration ( $P_s(t)$ ) (equation (4.23)) in response to a change in the phosphorus inflow concentration ( $P_o = P_{Lo}, t < 0$ ;  $P_o = P_{Loo}, t \geq 0$ ). The parameters  $X_1, X_2, X_3, X_4, X_5, X_6, X_7, \alpha, \phi$  and  $\gamma$  from the parameters program must be used.

USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter time (days)	t	ST000	t
2. Run program		E	$P_s(t)$
3. Enter time (days)	t	ST000	t
4. Run program		R/S	$P_s(t)$

000	76	LBL	060	54	)	120	53	(	180	43	RCL	240	43	RCL
001	16	A'	061	54	)	121	43	RCL	181	25	35	241	26	26
002	53	(	062	54	)	122	03	03	182	54	)	242	54	)
003	43	RCL	063	35	1/X	123	75	-	183	54	)	243	65	x
004	01	01	064	65	x	124	43	RCL	184	95	=	244	53	(
005	75	-	065	43	RCL	125	10	10	185	42	STD	245	43	RCL
006	43	RCL	066	06	06	126	54	)	186	27	27	246	31	21
007	04	04	067	95	=	127	54	)	187	53	(	247	65	x
008	54	)	068	42	STD	128	54	)	188	53	(	248	43	RCL
009	95	=	069	24	24	129	35	1/X	189	43	RCL	249	25	25
010	42	STD	070	92	RTN	130	65	x	190	26	26	250	75	-
011	21	21	071	76	LBL	131	43	RCL	191	75	-	251	43	RCL
012	92	RTN	072	10	E'	132	06	06	192	43	RCL	252	22	22
013	76	LBL	073	53	(	133	95	=	193	25	25	253	65	x
014	17	B'	074	43	RCL	134	42	STD	194	54	)	254	43	RCL
015	53	(	075	02	02	135	26	26	195	65	x	255	24	24
016	43	RCL	076	75	-	136	92	RTN	196	53	(	256	54	)
017	02	02	077	43	RCL	137	76	LBL	197	43	RCL	257	75	-
018	75	-	078	07	07	138	38	SIN	198	23	23	258	53	(
019	43	RCL	079	75	-	139	53	(	199	65	x	259	43	RCL
020	04	04	080	53	(	140	53	(	200	43	RCL	260	24	24
021	54	)	081	43	RCL	141	43	RCL	201	24	24	261	75	-
022	95	=	082	09	09	142	25	25	202	75	-	262	43	RCL
023	42	STD	083	65	x	143	75	-	203	43	RCL	263	25	25
024	22	22	084	43	RCL	144	43	RCL	204	21	21	264	54	)
025	92	RTN	085	10	10	145	24	24	205	65	x	265	65	x
026	76	LBL	086	55	+	146	54	)	206	43	RCL	266	53	(
027	18	C'	087	53	(	147	65	x	207	26	26	267	43	RCL
028	53	(	088	43	RCL	148	53	(	208	54	)	268	21	21
029	43	RCL	089	02	02	149	43	RCL	209	75	-	269	65	x
030	03	03	090	75	-	150	22	22	210	53	(	270	43	RCL
031	75	-	091	43	RCL	151	65	x	211	43	RCL	271	26	26
032	43	RCL	092	10	10	152	43	RCL	212	26	26	272	75	-
033	04	04	093	54	)	153	26	26	213	75	-	273	43	RCL
034	54	)	094	54	)	154	75	-	214	43	RCL	274	23	23
035	95	=	095	54	)	155	43	RCL	215	24	24	275	65	x
036	42	STD	096	35	1/X	156	23	23	216	54	)	276	43	RCL
037	23	23	097	65	x	157	65	x	217	65	x	277	24	24
038	92	RTN	098	43	RCL	158	43	RCL	218	53	(	278	54	)
039	76	LBL	099	06	06	159	25	25	219	43	RCL	279	54	)
040	19	D'	100	95	=	160	54	)	220	23	23	280	95	=
041	53	(	101	42	STD	161	75	-	221	65	x	281	42	STD
042	43	RCL	102	25	25	162	53	(	222	43	RCL	282	29	29
043	01	01	103	92	RTN	163	43	RCL	223	25	25	283	61	GTO
044	75	-	104	76	LBL	164	25	25	224	75	-	284	39	CDS
045	43	RCL	105	11	A	165	75	-	225	43	RCL	285	76	LBL
046	07	07	106	53	(	166	43	RCL	226	32	32	286	13	B
047	75	-	107	43	RCL	167	26	26	227	65	x	287	53	(
048	43	RCL	108	03	03	168	54	)	228	43	RCL	288	43	RCL
049	09	09	109	75	-	169	65	x	229	26	26	289	17	17
050	65	x	110	43	RCL	170	53	(	230	54	)	290	75	-
051	43	RCL	111	07	07	171	43	RCL	231	54	)	291	43	RCL
052	10	10	112	75	-	172	22	22	232	95	=	292	20	20
053	55	+	113	53	(	173	65	x	233	42	STD	293	54	)
054	53	(	114	43	RCL	174	43	RCL	234	28	28	294	65	x
055	43	RCL	115	09	09	175	24	24	235	53	(	295	53	(
056	01	01	116	65	x	176	75	-	236	53	(	296	53	(
057	75	-	117	43	RCL	177	43	RCL	237	43	RCL	297	43	RCL
058	43	RCL	118	10	10	178	21	21	238	24	24	298	01	01
059	10	10	119	55	+	179	65	x	239	75	-	299	65	x

Figure 33. Coding for TI-59 Programmable Calculator, lake model equation  $P_s(t)$  (4.23) solved by Trial Function Method.<sup>s</sup>

300	43	RCL	360	65	x	420	54	)	480	43	RCL	540	03	03
301	00	00	361	43	RCL	421	55	+	481	16	16	541	65	x
302	54	)	362	00	00	422	43	RCL	482	75	-	542	43	RCL
303	22	INV	363	54	)	423	27	27	483	43	RCL	543	00	00
304	23	LNx	364	22	INV	424	85	+	484	19	19	544	54	)
305	65	x	365	23	LNx	425	53	(	485	54	)	545	22	INV
306	43	RCL	366	65	x	426	43	RCL	486	94	+/-	546	23	LNx
307	25	25	367	43	RCL	427	02	02	487	65	x	547	65	x
308	65	x	368	24	24	428	65	x	488	53	(	548	43	RCL
309	53	(	369	65	x	429	43	RCL	489	53	(	549	24	24
310	43	RCL	370	53	(	430	00	00	490	43	RCL	550	65	x
311	22	22	371	43	RCL	431	54	)	491	01	01	551	53	(
312	65	x	372	21	21	432	22	INV	492	65	x	552	43	RCL
313	43	RCL	373	65	x	433	23	LNx	493	43	RCL	553	21	21
314	26	26	374	43	RCL	434	65	x	494	00	00	554	75	-
315	75	-	375	25	25	435	43	RCL	495	54	)	555	43	RCL
316	43	RCL	376	75	-	436	26	26	496	22	INV	556	22	22
317	23	23	377	43	RCL	437	65	x	497	23	LNx	557	54	)
318	65	x	378	24	24	438	53	(	498	65	x	558	55	+
319	43	RCL	379	65	x	439	43	RCL	499	43	RCL	559	43	RCL
320	25	25	380	43	RCL	440	26	26	500	25	25	560	29	29
321	54	)	381	22	22	441	75	-	501	65	x	561	54	)
322	55	+	382	54	)	442	43	RCL	502	53	(	562	54	)
323	43	RCL	383	55	+	443	24	24	503	43	RCL	563	95	=
324	27	27	384	43	RCL	444	54	)	504	22	22	564	92	RTN
325	85	+	385	29	29	445	55	+	505	75	-	565	76	LBL
326	53	(	386	54	)	446	43	RCL	506	43	RCL	566	15	E
327	43	RCL	387	54	)	447	28	28	507	23	23	567	16	A'
328	02	02	388	95	=	448	85	+	508	54	)	568	17	B'
329	65	x	389	92	RTN	449	53	(	509	55	+	569	18	C'
330	43	RCL	390	76	LBL	450	53	(	510	43	RCL	570	19	D'
331	00	00	391	13	C	451	43	RCL	511	27	27	571	10	E'
332	54	)	392	53	(	452	03	03	512	85	+	572	11	A
333	22	INV	393	43	RCL	453	65	x	513	53	(	573	61	GTO
334	23	LNx	394	15	15	454	43	RCL	514	43	RCL	574	38	SIN
335	65	x	395	75	-	455	00	00	515	02	02	575	76	LBL
336	43	RCL	396	43	RCL	456	54	)	516	65	x	576	39	CDS
337	26	26	397	18	18	457	22	INV	517	43	RCL	577	43	RCL
338	65	x	398	54	)	458	23	LNx	518	00	00	578	20	20
339	53	(	399	65	x	459	65	x	519	54	)	579	85	+
340	43	RCL	400	53	(	460	43	RCL	520	22	INV	580	12	B
341	23	23	401	53	(	461	24	24	521	23	LNx	581	85	+
342	65	x	402	43	RCL	462	65	x	522	65	x	582	13	C
343	43	RCL	403	01	01	463	53	(	523	43	RCL	583	95	+
344	24	24	404	65	x	464	43	RCL	524	26	26	584	14	D
345	75	-	405	43	RCL	465	24	24	525	65	x	585	95	=
346	43	RCL	406	00	00	466	75	-	526	53	(	586	91	R/S
347	21	21	407	54	)	467	43	RCL	527	43	RCL	587	61	GTO
348	65	x	408	22	INV	468	25	25	528	23	23	588	39	CDS
349	43	RCL	409	23	LNx	469	54	)	529	75	-			
350	26	26	410	65	x	470	55	+	530	43	RCL			
351	54	)	411	43	RCL	471	43	RCL	531	21	21			
352	55	+	412	25	25	472	29	29	532	54	)			
353	43	RCL	413	65	x	473	54	)	533	55	+			
354	28	28	414	53	(	474	54	)	534	43	RCL			
355	85	+	415	43	RCL	475	95	=	535	28	28			
356	53	(	416	25	25	476	92	RTN	536	85	+			
357	53	(	417	75	-	477	76	LBL	537	53	(			
358	43	RCL	418	43	RCL	478	14	D	538	53	(			
359	03	03	419	26	26	479	53	(	539	43	RCL			

Figure 33. (continued)

500.	00
-.0006305994	01
-.2596287671	02
-1.9726868817	03
-0.00175	04
2.976671851	05
.0020833333	06
-0.91	07
0.91	08
.0451962483	09
-.3211962483	10
0.	11
0.	12
0.	13
0.	14
90.	15
450.	16
375000.	17
50.	18
245.3	19
213000.	20
.0011194006	21
-.2578787671	22
-.9709368817	23
.0021822903	24
.0023509698	25
-.0245186247	26
.0000166436	27
-.0001735785	28
.0000154494	29

Figure 33. (continued)



Equations (4.29, 30 and 31) obtained by approximation techniques.

## LAKE WATER PHOSPHORUS CONCENTRATION

Approximate Equation

The following program calculates the lake water phosphorus concentration ( $P_L(t)$ ) (equation (4.31)) in response to a change in the phosphorus inflow concentration ( $P_o = P_{Lo}$ ,  $t < 0$ ;  $P_o = P_{L\infty}$ ,  $t \geq 0$ ).

## USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter Parameters	$P_{io}$	STO08	$P_{io}$
	$P_{so}$	STO09	$P_{so}$
	$P_o$	STO10	$P_o$
	$K_1$	STO11	$K_1$
	$K_2$	STO12	$K_2$
	$K_3$	STO13	$K_3$
	Z	STO14	Z
	Dr	STO15	Dr
	T	STO16	T
2. Enter time (days)	$\mathcal{E}$	STO17	$\mathcal{E}$
	t	STO00	t
3. Run program		E	$P_L(t)$
4. Enter time (days)	t	STO00	t
5. Run program		R/S	$P_L(t)$

000	76	LBL	060	14	14	120	55	+	180	43	RCL	240	00	00
001	11	A	061	33	X <sup>2</sup>	121	43	RCL	181	00	00	241	54	)
002	53	(	062	65	x	122	18	18	182	54	)	242	22	INV
003	43	RCL	063	43	RCL	123	94	+/-	183	22	INV	243	23	LNx
004	14	14	064	10	10	124	54	)	184	23	LNx	244	65	x
005	65	x	065	65	x	125	42	STD	185	65	x	245	53	(
006	43	RCL	066	43	RCL	126	06	06	186	53	(	246	53	(
007	15	15	067	12	12	127	53	(	187	53	(	247	53	(
008	85	+	068	55	+	128	53	(	188	53	(	248	53	(
009	43	RCL	069	43	RCL	129	53	(	189	53	(	249	43	RCL
010	16	16	070	18	18	130	43	RCL	190	43	RCL	250	05	05
011	65	x	071	94	+/-	131	05	05	191	05	05	251	85	+
012	43	RCL	072	54	)	132	75	-	192	85	+	252	43	RCL
013	15	15	073	42	STD	133	43	RCL	193	43	RCL	253	01	01
014	65	x	074	03	03	134	01	01	194	01	01	254	54	)
015	53	(	075	53	(	135	54	)	195	54	)	255	55	+
016	43	RCL	076	43	RCL	136	33	X <sup>2</sup>	196	94	+/-	256	02	2
017	17	17	077	13	13	137	75	-	197	55	+	257	94	+/-
018	65	x	078	55	+	138	04	4	198	02	2	258	75	-
019	43	RCL	079	43	RCL	139	65	x	199	85	+	259	43	RCL
020	11	11	080	17	17	140	53	(	200	43	RCL	260	07	07
021	85	+	081	54	)	141	43	RCL	201	07	07	261	54	)
022	43	RCL	082	42	STD	142	02	02	202	54	)	262	65	x
023	12	12	083	04	04	143	65	x	203	65	x	263	43	RCL
024	65	x	084	53	(	144	43	RCL	204	43	RCL	264	08	08
025	43	RCL	085	53	(	145	04	04	205	08	08	265	85	+
026	14	14	086	43	RCL	146	75	-	206	85	+	266	43	RCL
027	54	)	087	11	11	147	43	RCL	207	43	RCL	267	04	04
028	54	)	088	33	X <sup>2</sup>	148	01	01	208	09	09	268	65	x
029	42	STD	089	65	x	149	65	x	209	65	x	269	43	RCL
030	18	18	090	43	RCL	150	43	RCL	210	43	RCL	270	09	09
031	43	RCL	091	17	17	151	05	05	211	04	04	271	54	)
032	13	13	092	65	x	152	54	)	212	54	)	272	55	+
033	94	+/-	093	43	RCL	153	54	)	213	55	+	273	43	RCL
034	42	STD	094	16	16	154	34	FX	214	43	RCL	274	07	07
035	01	01	095	55	+	155	55	+	215	07	07	275	94	+/-
036	53	(	096	43	RCL	156	02	2	216	55	+	276	55	+
037	43	RCL	097	18	18	157	54	)	217	02	2	277	02	2
038	17	17	098	94	+/-	158	42	STD	218	54	)	278	54	)
039	65	x	099	54	)	159	07	07	219	92	RTN	279	92	RTN
040	43	RCL	100	85	+	160	92	RTN	220	76	LBL	280	76	LBL
041	11	11	101	53	(	161	76	LBL	221	17	B*	281	13	C
042	65	x	102	43	RCL	162	12	B	222	53	(	282	53	(
043	43	RCL	103	11	11	163	53	(	223	53	(	283	53	(
044	12	12	104	55	+	164	53	(	224	53	(	284	53	(
045	65	x	105	43	RCL	165	53	(	225	43	RCL	285	53	(
046	43	RCL	106	15	15	166	43	RCL	226	05	05	286	53	(
047	14	14	107	54	)	167	05	05	227	75	-	287	43	RCL
048	65	x	108	54	)	168	75	-	228	43	RCL	288	05	05
049	43	RCL	109	42	STD	169	43	RCL	229	01	01	289	75	-
050	16	16	110	05	05	170	01	01	230	54	)	290	43	RCL
051	55	+	111	53	(	171	54	)	231	94	+/-	291	01	01
052	43	RCL	112	43	RCL	172	94	+/-	232	55	+	292	54	)
053	18	18	113	10	10	173	55	+	233	02	2	293	55	+
054	94	+/-	114	65	x	174	02	2	234	75	-	294	02	2
055	54	)	115	43	RCL	175	85	+	235	43	RCL	295	94	+/-
056	42	STD	116	11	11	176	43	RCL	236	07	07	296	75	-
057	02	02	117	65	x	177	07	07	237	54	)	297	43	RCL
058	53	(	118	43	RCL	178	54	)	238	65	x	298	07	07
059	43	RCL	119	14	14	179	65	x	239	43	RCL	299	54	)

Figure 34. Coding for TI-59 Programmable Calculator, lake model equation  $P_L(t)$  solved by Approximation techniques.

300	65	x	360	92	RTN	420	07	07	480	54	)	
301	43	RCL	361	76	LBL	421	55	+	481	95	=	
302	00	00	362	18	C'	422	02	2	482	91	R/S	
303	54	)	363	53	(	423	55	+	483	61	GTO	
304	22	INV	364	53	(	424	53	(	484	10	E'	
305	23	LNK	365	53	(	425	53	(		1000.		00
306	94	+/-	366	53	(	426	43	RCL		-0.001		01
307	85	+	367	53	(	427	05	05		-.4188091954		02
308	01	1	368	43	RCL	428	75	-		-46.29693487		03
309	54	)	369	05	05	429	43	RCL		.0011904762		04
310	55	x	370	75	-	430	01	01		.7818678161		05
311	53	(	371	43	RCL	431	54	)		-14.16427203		06
312	53	(	372	01	01	432	94	+/-		.3910718846		07
313	53	(	373	54	)	433	55	+		440.5		08
314	43	RCL	374	94	+/-	434	02	2		267690.		09
315	05	05	375	55	+	435	85	+		50.		10
316	85	+	376	02	2	436	43	RCL		0.091		11
317	43	RCL	377	85	+	437	07	07		0.176		12
318	01	01	378	43	RCL	438	54	)		0.001		13
319	54	)	379	07	07	439	54	)		1.69		14
320	55	+	380	54	)	440	92	RTN		0.1		15
321	02	2	381	65	x	441	76	LBL		10.		16
322	94	+/-	382	43	RCL	442	15	E		0.84		17
323	75	-	383	00	00	443	11	A		0.54288		18
324	43	RCL	384	54	)	444	76	LBL		0.		19
325	07	07	385	22	INV	445	10	E'		0.		20
326	54	)	386	23	LNK	446	53	(		0.		21
327	65	x	387	94	+/-	447	12	B		0.		22
328	43	RCL	388	85	+	448	85	+		0.		23
329	06	06	389	01	1	449	17	B'		0.		24
330	85	+	390	54	)	450	85	+		0.		25
331	43	RCL	391	65	x	451	13	C		0.		26
332	04	04	392	53	(	452	85	+		0.		27
333	65	x	393	53	(	453	18	C'		0.		28
334	43	RCL	394	53	(	454	54	)		0.		29
335	03	03	395	43	RCL	455	65	x		0.		30
336	54	)	396	05	05	456	53	(		0.		31
337	55	+	397	85	+	457	43	RCL		0.		32
338	43	RCL	398	43	RCL	458	02	02		0.		33
339	07	07	399	01	01	459	65	x		0.		34
340	55	+	400	54	)	460	43	RCL		0.		35
341	02	2	401	94	+/-	461	15	15		0.		36
342	55	+	402	55	+	462	55	+		0.		37
343	53	(	403	02	2	463	43	RCL		0.		38
344	53	(	404	85	+	464	12	12		0.		39
345	43	RCL	405	43	RCL	465	55	+				
346	05	05	406	07	07	466	43	RCL				
347	75	-	407	54	)	467	14	14				
348	43	RCL	408	65	x	468	94	+/-				
349	01	01	409	43	RCL	469	54	)				
350	54	)	410	06	06	470	75	-				
351	94	+/-	411	85	+	471	53	(				
352	55	+	412	43	RCL	472	43	RCL				
353	02	2	413	04	04	473	06	06				
354	75	-	414	65	x	474	65	x				
355	43	RCL	415	43	RCL	475	43	RCL				
356	07	07	416	03	03	476	15	15				
357	54	)	417	54	)	477	55	+				
358	54	)	418	55	+	478	43	RCL				
359	94	+/-	419	43	RCL	479	11	11				

Figure 34. (continued)

## SEDIMENT INTERSTITIAL PHOSPHORUS CONCENTRATION

Approximate Equation

The following program calculates the sediment interstitial phosphorus concentration ( $P_i(t)$ ) (equation (4.29)) in response to a change in phosphorus inflow concentration ( $P_o = P_{Lo}$ ,  $t < 0$ ;  $P_o = P_{L\infty}$ ,  $t \geq 0$ ).

## USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter Parameters	$P_{io}$	STO08	$P_{io}$
	$P_{so}$	STO09	$P_{so}$
	$P_o$	STO10	$P_o$
	$K_1$	STO11	$K_1$
	$K_2$	STO12	$K_2$
	$K_3$	STO13	$K_3$
	Z	STO14	Z
	Dr	STO15	Dr
	T	STO16	T
2. Enter time (days)	$\mathcal{E}$	STO17	$\mathcal{E}$
	t	STO00	t
	3. Run program	E	$P_i(t)$
	4. Enter time (days)	t	t
	5. Run program	R/S	$P_i(t)$

000	76	LBL	060	14	14	120	55	+	130	43	RCL	240	00	00
001	11	R	061	33	X2	121	43	RCL	131	00	00	241	54	)
002	53	(	062	65	x	122	18	18	132	54	)	242	22	INV
003	43	RCL	063	43	RCL	123	94	+/-	133	22	INV	243	23	LNx
004	14	14	064	10	10	124	54	)	134	23	LNx	244	65	x
005	65	x	065	65	x	125	42	STD	135	65	x	245	53	(
006	43	RCL	066	43	RCL	126	06	06	136	53	(	246	53	(
007	15	15	067	12	12	127	53	(	137	53	(	247	53	(
008	85	+	068	55	+	128	53	(	138	53	(	248	53	(
009	43	RCL	069	43	RCL	129	53	(	139	53	(	249	43	RCL
010	16	16	070	18	18	130	43	RCL	190	43	RCL	250	05	05
011	65	x	071	94	+/-	131	05	05	191	05	05	251	85	+
012	43	RCL	072	54	)	132	75	-	192	85	+	252	43	RCL
013	15	15	073	42	STD	133	43	RCL	193	43	RCL	253	01	01
014	65	x	074	03	03	134	01	01	194	01	01	254	54	)
015	53	(	075	53	(	135	54	)	195	54	)	255	55	+
016	43	RCL	076	43	RCL	136	33	X2	196	94	+/-	256	02	2
017	17	17	077	13	13	137	75	-	197	55	+	257	94	+/-
018	65	x	078	55	+	138	04	4	198	02	2	258	75	-
019	43	RCL	079	43	RCL	139	65	x	199	85	+	259	43	RCL
020	11	11	080	17	17	140	53	(	200	43	RCL	260	07	07
021	85	+	081	54	)	141	43	RCL	201	07	07	261	54	)
022	43	RCL	082	42	STD	142	02	02	202	54	)	262	65	x
023	12	12	083	04	04	143	65	x	203	65	x	263	43	RCL
024	65	x	084	53	(	144	43	RCL	204	43	RCL	264	08	08
025	43	RCL	085	53	(	145	04	04	205	08	08	265	85	+
026	14	14	086	43	RCL	146	75	-	206	85	+	266	43	RCL
027	54	)	087	11	11	147	43	RCL	207	43	RCL	267	04	04
028	54	)	088	33	X2	148	01	01	208	09	09	268	65	x
029	42	STD	089	65	x	149	65	x	209	65	x	269	43	RCL
030	18	18	090	43	RCL	150	43	RCL	210	43	RCL	270	09	09
031	43	RCL	091	17	17	151	05	05	211	04	04	271	54	)
032	13	13	092	65	x	152	54	)	212	54	)	272	55	+
033	94	+/-	093	43	RCL	153	54	)	213	55	+	273	43	RCL
034	42	STD	094	16	16	154	34	FX	214	43	RCL	274	07	07
035	01	01	095	55	+	155	55	+	215	07	07	275	94	+/-
036	53	(	096	43	RCL	156	02	2	216	55	+	276	55	+
037	43	RCL	097	18	18	157	54	)	217	02	2	277	02	2
038	17	17	098	94	+/-	158	42	STD	218	54	)	278	54	)
039	65	x	099	54	)	159	07	07	219	92	RTN	279	92	RTN
040	43	RCL	100	85	+	160	92	RTN	220	76	LBL	280	76	LBL
041	11	11	101	53	(	161	76	LBL	221	17	8'	281	13	C
042	65	x	102	43	RCL	162	12	B	222	53	(	282	53	(
043	43	RCL	103	11	11	163	53	(	223	53	(	283	53	(
044	12	12	104	55	+	164	53	(	224	53	(	284	53	(
045	65	x	105	43	RCL	165	53	(	225	43	RCL	285	53	(
046	43	RCL	106	15	15	166	43	RCL	226	05	05	286	53	(
047	14	14	107	54	)	167	05	05	227	75	-	287	43	RCL
048	65	x	108	54	)	168	75	-	228	43	RCL	288	05	05
049	43	RCL	109	42	STD	169	43	RCL	229	01	01	289	75	-
050	16	16	110	05	05	170	01	01	230	54	)	290	43	RCL
051	55	+	111	53	(	171	54	)	231	94	+/-	291	01	01
052	43	RCL	112	43	RCL	172	94	+/-	232	55	+	292	54	)
053	18	18	113	10	10	173	55	+	233	02	2	293	55	+
054	94	+/-	114	65	x	174	02	2	234	75	-	294	02	2
055	54	)	115	43	RCL	175	85	+	235	43	RCL	295	94	+/-
056	42	STD	116	11	11	176	43	RCL	236	07	07	296	75	-
057	02	02	117	65	x	177	07	07	237	54	)	297	43	RCL
058	53	(	118	43	RCL	178	54	)	238	65	x	298	07	07
059	43	RCL	119	14	14	179	65	x	239	43	RCL	299	54	)

Figure 35. Coding for TI-59 Programmable Calculator, lake model equation  $P_1(t)$  solved by Approximation techniques.

300	65	x	360	92	RTN	420	07	07
301	43	RCL	361	76	LBL	421	55	+
302	00	00	362	18	C'	422	02	2
303	54	)	363	53	(	423	55	+
304	22	INV	364	53	(	424	53	(
305	23	LNx	365	53	(	425	53	(
306	94	+/-	366	53	(	426	43	RCL
307	85	+	367	53	(	427	05	05
308	01	1	368	43	RCL	428	75	-
309	54	)	369	05	05	429	43	RCL
310	65	x	370	75	-	430	01	01
311	53	(	371	43	RCL	431	54	)
312	53	(	372	01	01	432	94	+/-
313	53	(	373	54	)	433	55	+
314	43	RCL	374	94	+/-	434	02	2
315	05	05	375	55	+	435	85	+
316	85	+	376	02	2	436	43	RCL
317	43	RCL	377	85	+	437	07	07
318	01	01	378	43	RCL	438	54	)
319	54	)	379	07	07	439	54	)
320	55	+	380	54	)	440	92	RTN
321	02	2	381	65	x	441	76	LBL
322	94	+/-	382	43	RCL	442	15	E
323	75	-	383	00	00	443	11	A
324	43	RCL	384	54	)	444	76	LBL
325	07	07	385	22	INV	445	10	E'
326	54	)	386	23	LNx	446	53	(
327	65	x	387	94	+/-	447	12	B
328	43	RCL	388	85	+	448	85	+
329	06	06	389	01	1	449	17	B'
330	85	+	390	54	)	450	85	+
331	43	RCL	391	65	x	451	13	C
332	04	04	392	53	(	452	85	+
333	65	x	393	53	(	453	18	C'
334	43	RCL	394	53	(	454	54	)
335	03	03	395	43	RCL	455	91	R/S
336	54	)	396	05	05	456	61	GTO
337	55	+	397	85	+	457	10	E'
338	43	RCL	398	43	RCL		0.	00
339	07	07	399	01	01		-0.001	01
340	55	+	400	54	)		-4188091954	02
341	02	2	401	94	+/-		-46.29633487	03
342	55	+	402	55	+		.0011904762	04
343	53	(	403	02	2		.7818678161	05
344	53	(	404	85	+		-14.16427203	06
345	43	RCL	405	43	RCL		.3910718846	07
346	05	05	406	07	07		450.	08
347	75	-	407	54	)		267690.	09
348	43	RCL	408	65	x		50.	10
349	01	01	409	43	RCL		0.091	11
350	54	)	410	06	06		0.176	12
351	94	+/-	411	85	+		0.001	13
352	55	+	412	43	RCL		1.69	14
353	02	2	413	04	04		0.1	15
354	75	-	414	65	x		10.	16
355	43	RCL	415	43	RCL		0.84	17
356	07	07	416	03	03		0.54288	18
357	54	)	417	54	)			
358	54	)	418	55	+			
359	94	+/-	419	43	RCL			

Figure 35. (continued)

## SEDIMENT SOLID PHASE PHOSPHORUS CONCENTRATION

Approximate Equation

This program calculates the sediment solid phase phosphorus concentration ( $P_s(t)$ ) (equation (4.30)) in response to a change in phosphorus inflow concentration ( $P_o = P_{Lo}$ ,  $t < 0$ ;  $P_o = P_{L\infty}$ ,  $t \geq 0$ ).

## USER INSTRUCTIONS

<u>Procedure</u>	<u>Enter</u>	<u>Press</u>	<u>Display</u>
1. Enter Parameters	A	STO10	A
	$K_1$	STO11	$K_1$
	$K_2$	STO12	$K_2$
	$K_3$	STO13	$K_3$
	Q	STO14	Q
	$V_L$	STO15	$V_L$
	$V_s$	STO16	$V_s$
	$\mathcal{E}$	STO17	$\mathcal{E}$
	$P_o$	STO18	$P_o$
	$P_{io}$	STO19	$P_{io}$
	$P_{so}$	STO20	$P_{so}$
2. Enter time (days)	t	STO00	t
3. Run program		E	$P_i(t)$
4. Enter time (days)	t	STO00	t
5. Run program		R/S	$P_i(t)$



000	76	LBL	060	06	06	120	07	07	180	07	07	240	00	0
001	11	R	061	53	(	121	54	)	181	85	+	241	54	)
002	43	RCL	062	53	(	122	33	X <sup>2</sup>	182	43	RCL	242	22	INV
003	13	13	063	43	RCL	123	75	-	183	01	01	243	23	LN <sub>X</sub>
004	94	+/-	064	14	14	124	04	4	184	65	x	244	65	x
005	42	STD	065	85	+	125	65	x	185	43	RCL	245	53	(
006	01	01	066	43	RCL	126	53	(	186	07	07	246	53	(
007	53	(	067	10	10	127	43	RCL	187	54	)	247	53	(
008	43	RCL	068	65	x	128	02	02	188	55	+	248	53	(
009	12	12	069	43	RCL	129	65	x	189	02	2	249	43	RCL
010	65	x	070	11	11	130	43	RCL	190	55	+	250	05	05
011	43	RCL	071	85	+	131	06	06	191	43	RCL	251	85	+
012	15	15	072	43	RCL	132	65	x	192	07	07	252	43	RCL
013	55	+	073	12	12	133	43	RCL	193	54	)	253	01	01
014	43	RCL	074	65	x	134	03	03	194	42	STD	254	54	)
015	16	16	075	43	RCL	135	55	+	195	21	21	255	55	+
016	54	)	076	15	15	136	43	RCL	196	53	(	256	02	2
017	42	STD	077	54	)	137	07	07	197	53	(	257	94	+/-
018	02	02	078	94	+/-	138	75	-	198	43	RCL	258	75	-
019	53	(	079	55	+	139	43	RCL	199	05	05	259	43	RCL
020	43	RCL	080	43	RCL	140	01	01	200	65	x	260	07	07
021	13	13	081	15	15	141	65	x	201	43	RCL	261	54	)
022	55	+	082	54	)	142	53	(	202	06	06	262	65	x
023	43	RCL	083	42	STD	143	53	(	203	75	-	263	43	RCL
024	17	17	084	07	07	144	43	RCL	204	43	RCL	264	08	08
025	54	)	085	53	(	145	05	05	205	04	04	265	85	+
026	42	STD	086	43	RCL	146	65	x	206	65	x	266	43	RCL
027	03	03	087	18	18	147	43	RCL	207	43	RCL	267	04	04
028	53	(	088	65	x	148	06	06	208	07	07	268	65	x
029	43	RCL	089	43	RCL	149	75	-	209	75	-	269	43	RCL
030	10	10	090	14	14	150	43	RCL	210	43	RCL	270	09	09
031	94	+/-	091	55	+	151	04	04	211	01	01	271	54	)
032	65	x	092	43	RCL	152	65	x	212	65	x	272	55	+
033	43	RCL	093	15	15	153	43	RCL	213	43	RCL	273	43	RCL
034	11	11	094	54	)	154	07	07	214	07	07	274	07	07
035	55	+	095	42	STD	155	54	)	215	54	)	275	94	+/-
036	43	RCL	096	08	08	156	55	+	216	55	+	276	55	+
037	16	16	097	53	(	157	43	RCL	217	02	2	277	02	2
038	54	)	098	53	(	158	07	07	218	55	+	278	54	)
039	42	STD	099	53	(	159	54	)	219	43	RCL	279	92	RTN
040	04	04	100	43	RCL	160	54	)	220	07	07	280	76	LBL
041	43	RCL	101	05	05	161	54	)	221	54	)	281	13	C
042	04	04	102	65	x	162	34	FX	222	42	STD	282	53	(
043	94	+/-	103	43	RCL	163	55	+	223	22	22	283	53	(
044	42	STD	104	06	06	164	02	2	224	92	RTN	284	53	(
045	05	05	105	75	-	165	54	)	225	76	LBL	285	53	(
046	53	(	106	43	RCL	166	42	STD	226	10	E'	286	53	(
047	43	RCL	107	04	04	167	23	23	227	53	(	287	43	RCL
048	17	17	108	65	x	168	53	(	228	53	(	288	05	05
049	65	x	109	43	RCL	169	53	(	229	53	(	289	75	-
050	43	RCL	110	07	07	170	43	RCL	230	43	RCL	290	43	RCL
051	10	10	111	75	-	171	05	05	231	22	22	291	01	01
052	65	x	112	43	RCL	172	65	x	232	75	-	292	54	)
053	43	RCL	113	01	01	173	43	RCL	233	43	RCL	293	55	+
054	11	11	114	65	x	174	06	06	234	23	23	294	02	2
055	55	+	115	43	RCL	175	75	-	235	54	)	295	94	+/-
056	43	RCL	116	07	07	176	43	RCL	236	65	x	296	75	-
057	15	15	117	54	)	177	04	04	237	43	RCL	297	43	RCL
058	54	)	118	55	+	178	65	x	238	00	00	298	07	07
059	42	STD	119	43	RCL	179	43	RCL	239	94	+/-	299	54	)

Figure 36. Coding for TI-59 Programmable Calculator, lake model equation  $P_s(t)$  solved by Approximation techniques.

300	21	21	360	05	05	420	43	RCL	5.620689655	08
301	75	-	361	55	+	421	08	08	0.	09
302	43	RCL	362	43	RCL	422	65	x	257200.	10
303	23	23	363	07	07	423	53	(	0.091	11
304	54	)	364	33	X²	424	43	RCL	0.176	12
305	75	-	365	75	-	425	06	06	0.001	13
306	53	(	366	01	1	426	65	x	48900.	14
307	43	RCL	367	54	)	427	43	RCL	435000.	15
308	19	19	368	65	x	428	05	05	25720.	16
309	65	x	369	53	(	429	55	+	0.84	17
310	43	RCL	370	43	RCL	430	43	RCL	50.	18
311	02	02	371	21	21	431	07	07	440.5	19
312	65	x	372	75	-	432	33	X²	267690.	20
313	43	RCL	373	43	RCL	433	75	-	.3944089415	21
314	06	06	374	23	23	434	01	1	.3954089415	22
315	55	+	375	54	)	435	54	)	.3950017946	23
316	43	RCL	376	55	+	436	65	x		
317	07	07	377	02	2	437	53	(		
318	54	)	378	55	+	438	43	RCL		
319	54	)	379	43	RCL	439	21	21		
320	55	+	380	23	23	440	85	+		
321	02	2	381	55	+	441	43	RCL		
322	55	+	382	53	(	442	23	23		
323	43	RCL	383	43	RCL	443	54	)		
324	23	23	384	22	22	444	55	+		
325	54	)	385	85	+	445	02	2		
326	75	-	386	43	RCL	446	55	+		
327	53	(	387	23	23	447	43	RCL		
328	53	(	388	54	)	448	23	23		
329	53	(	389	55	+	449	55	+		
330	43	RCL	390	43	RCL	450	53	(		
331	22	22	391	07	07	451	43	RCL		
332	85	+	392	54	)	452	22	22		
333	43	RCL	393	54	)	453	75	-		
334	23	23	394	85	+	454	43	RCL		
335	54	)	395	53	(	455	23	23		
336	65	x	396	53	(	456	54	)		
337	43	RCL	397	53	(	457	55	+		
338	00	00	398	43	RCL	458	43	RCL		
339	94	+/-	399	22	22	459	07	07		
340	54	)	400	75	-	460	54	)		
341	22	INV	401	43	RCL	461	54	)		
342	23	LNx	402	23	23	462	54	)		
343	94	+/-	403	54	)	463	54	)		
344	85	+	404	65	x	464	92	RTN		
345	01	1	405	43	RCL	465	76	LBL		
346	54	)	406	00	00	466	15	E		
347	65	x	407	94	+/-	467	11	A		
348	53	(	408	54	)	468	10	E*		
349	43	RCL	409	22	INV	469	91	R/S		
350	02	02	410	23	LNx	470	61	GTD		
351	65	x	411	94	+/-	471	10	E*		
352	43	RCL	412	85	+				0.	00
353	08	08	413	01	1				-0.001	01
354	65	x	414	54	)				2.976671851	02
355	53	(	415	65	x				.0011904762	03
356	43	RCL	416	53	(				-0.91	04
357	06	06	417	43	RCL				0.91	05
358	65	x	418	02	02				.0451962483	06
359	43	RCL	419	65	x				-1.3422138506	07

Figure 36. (continued)